

DEVELOPMENT APPLICATION

APPLICATION NUMBER:	PLN-24-330
PROPOSED DEVELOPMENT:	Demolition and Change of Use from Residential to Manufacturing & Processing
LOCATION:	115 Howard Road Goodwood
APPLICANT:	Island Life Designers
ADVERTISING START DATE:	12/02/2025
ADVERTISING EXPIRY DATE:	4/03/2025

Plans and documentation are available for inspection at Council's Offices, located at 374 Main Road, Glenorchy between 8.30 am and 5.00 pm, Monday to Friday (excluding public holidays) and the plans are available on Glenorchy City Council's website (<u>www.gcc.tas.gov.au</u>) until **4/03/2025**.

During this time, any person may make representations relating to the applications by letter addressed to the Chief Executive Officer, Glenorchy City Council, PO Box 103, Glenorchy 7010 or by email to gccmail@gcc.tas.gov.au.

Representations must be received by no later than 11.59 pm on **4/03/2025**, or for postal and hand delivered representations, by 5.00 pm on **4/03/2025**.

GENERAL INFORMATION

Land Title Referene	59719/5
Building Class	TBC
Property Zone	Light Industrial
Wind Classification	N2
Soil Classification (AS 2870)	CLASS P
Climate Zone (NCC 3.12)	Zone 7
Alpine Area (900m above AHD) -	NA
BAL Rating (AS3959)	NA
Heritage Building	NO
Flood Prone Area	TBC
Coastal Ingress Area	NO
Coastal Erosion Area	NO
Corrosion Environment	Moderate

OTHER CONSULTANTS		
Structural Engineer	-	T.B.C
Geological Report (Soil)	-	GES
Energy Assessment	-	T.B.C
Waste Water Report		NA
Bushfire Assessment	-	NA
Civil Engineer	-	TBC
Mechanical Engineer	-	NA
Electrical Engineer	-	NA
Site Survey	-	Lark & Creese Land & Engineering Surveyors
Hydrologist Report		ΝΑ
Contaminated Site Survey	-	NA
- · · · · · · · · · · · · · · · · · · ·		
AREA SCHEDULE		(All measurements in m2)
AREA SCHEDULE Site Plan		(All measurements in m2) 827m2
	-	
Site Plan	-	827m2
Site Plan Existing Residence	-	827m2 NA
Site Plan Existing Residence Residence (Ground Floor)	-	827m2 NA NA
Site Plan Existing Residence Residence (Ground Floor) Residence (First Floor)		827m2 NA NA NA
Site Plan Existing Residence Residence (Ground Floor) Residence (First Floor) Alfresco Area		827m2 NA NA NA NA
Site Plan Existing Residence Residence (Ground Floor) Residence (First Floor) Alfresco Area Verandah Area		827m2 NA NA NA NA
Site Plan Existing Residence Residence (Ground Floor) Residence (First Floor) Alfresco Area Verandah Area Porch (Laundry)		827m2 NA NA NA NA NA
Site Plan Existing Residence Residence (Ground Floor) Residence (First Floor) Alfresco Area Verandah Area Porch (Laundry) Porch		827m2 NA NA NA NA NA NA
Site Plan Existing Residence Residence (Ground Floor) Residence (First Floor) Alfresco Area Verandah Area Porch (Laundry) Porch Decking Area	-	827m2 NA NA NA NA NA NA NA
Site Plan Existing Residence Residence (Ground Floor) Residence (First Floor) Alfresco Area Verandah Area Porch (Laundry) Porch Decking Area Balcony (existing)		827m2 NA NA NA NA NA NA NA NA

GLENORCHY CITY COUNCIL PLANNING SERVICES			
APPLICATION No .	PLN-24-330		
DATE RECEIVED	28-11-2024		

IG SERVICES	
PLN-24-330	
28-11-2024	

Sheet numbe

09 10 11

Proposed Manufacturing & Sales Warehouse

No. 115 Howard Road Goodwood TAS 7010



ISLAND LIFE DESIGNERS				PROJECT NAME :
BUILDING SERVICES PROVIDER LICENCE No. 456943679	Concept Layout	A	stage sketch design	Proposed Manufacturing
CONTACT: nick@islandlifedesigners.com	New Driveway/ crossover/ carparking	B	preliminary design	
			contract documentation	
General Notes				
The Builder shall check all dimensions and levels on site prior to construction. Notify any errors, discrepancies or omissions to the building designer.			ВА	CLIENT : Mr. M & S Askev
Drawings shall not be used for construction purposes until issued for construction.			acception drawings	WIT. WICH O ASKEY

bsed Manufacturing and Sales Warehouse

115 Howard Road Goodwoo TAS 7010 DRAWING TITLE : Title

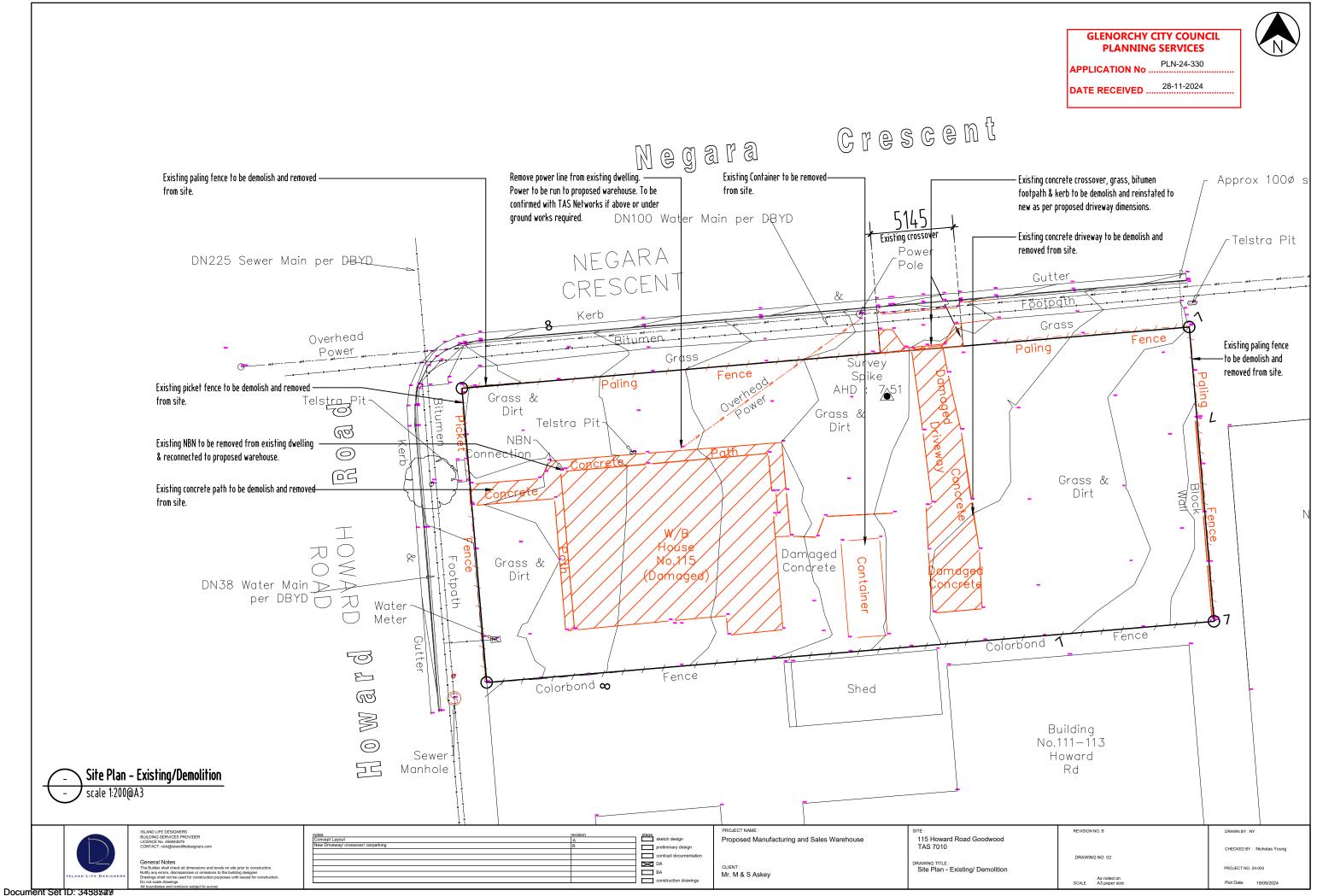
Document Set ID: 3458529 Version: 2, Version Date: 28/01/2025

	Dra	awing Schedule:		
ſ		Sheet name		
	TITLE F	PAGE		
	SITE PL	AN – EXISTING/ DEMOLITION		
	SITE PL	AN		
	LANDS	CAPE PLAN		
	SITE PL	.AN – DRAINAGE		
	FLOOR	PLAN		
	ROOF P	LAN		
	GROUN	D FLOOR DRAINAGE PLAN		
	ELEVA	TIONS 1 & 2		
	ELEVA	TIONS 3 & 4		
	SHADO	W DIAGRAM		
		REVISION NO. B	DRAWN BY : NY	

As noted on A3 paper size	Plot Date: 18/09/2024
	PROJECT NO. 24-003
VING NO 01	CHECKED BY : Nicholas Young

DRA\

SCALE



Certificate of Title: 59719/5 115 Howard Road Goodwood TAS 7010 Site Area: 827m2 Proposed Shed Cover 293.41m2 Total Site Cover: 35.47%

LEGEND & NOTES

-+ Finished Floor Levels

Note: Site levels as per survey Soil & Water Management Strategies

Downpipes to be connected as soon as the roof is installed.

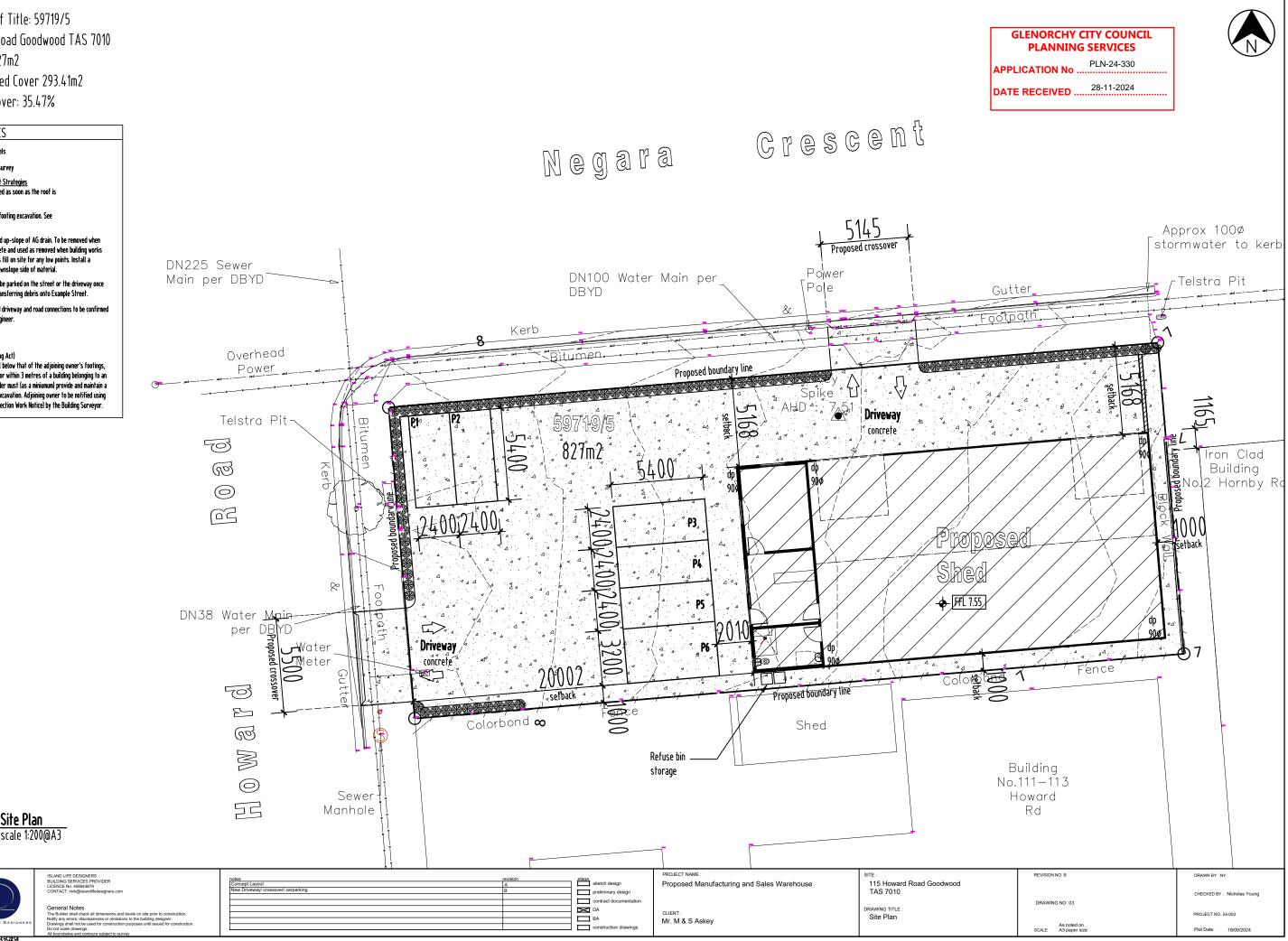
Install AG drain prior to footing excavation. See drawing

Excavated material placed up-slope of AG drain. To be removed when building works are complete and used as removed when building works are complete and used as fill on site for any low points. Install a sediment fence on the downslope side of material

Construction vehicles to be parked on the street or the driveway once concreted, to prevent transferring debris onto Example Street.

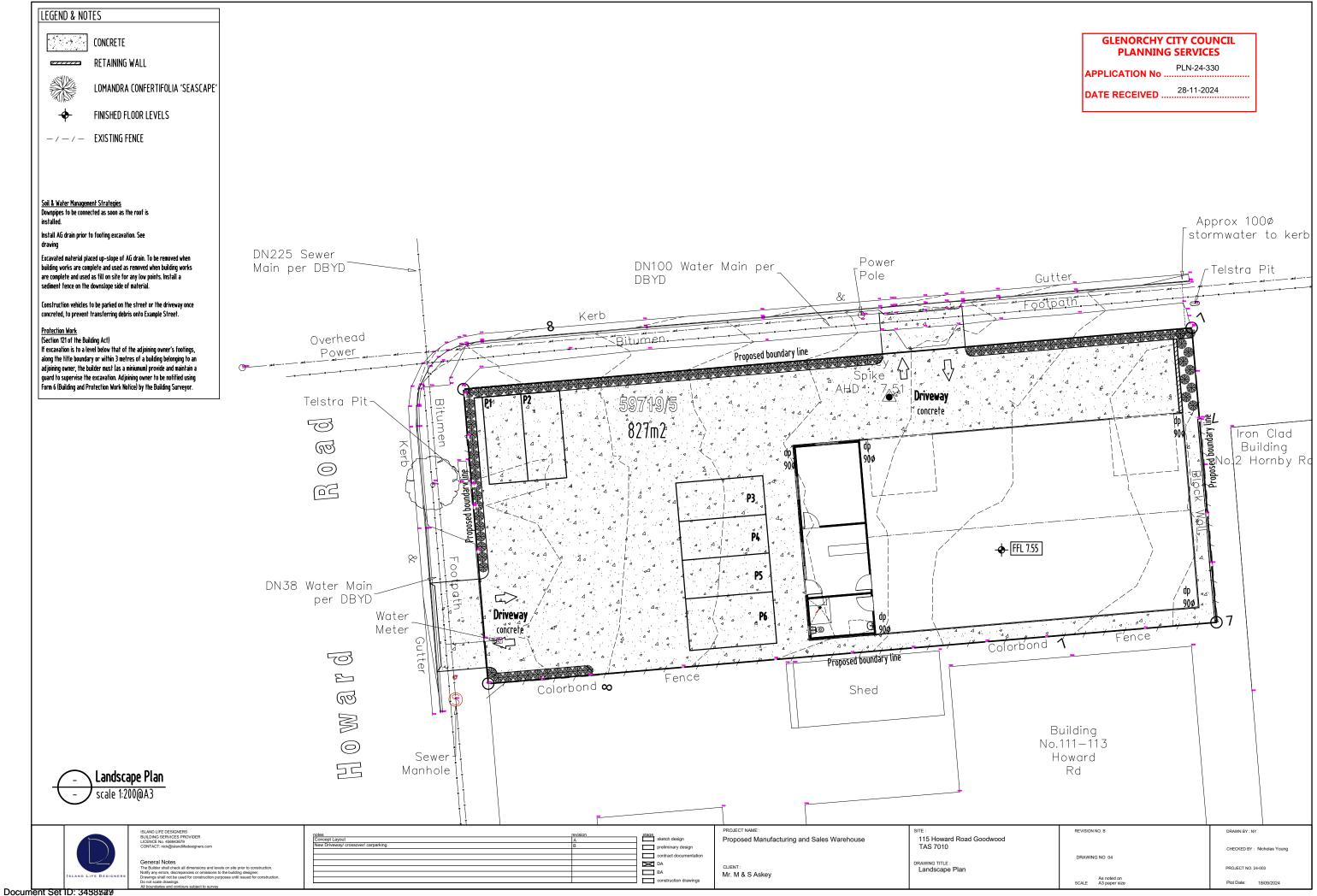
All existing and proposed driveway and road connections to be confirmed and completed by Civil engineer.

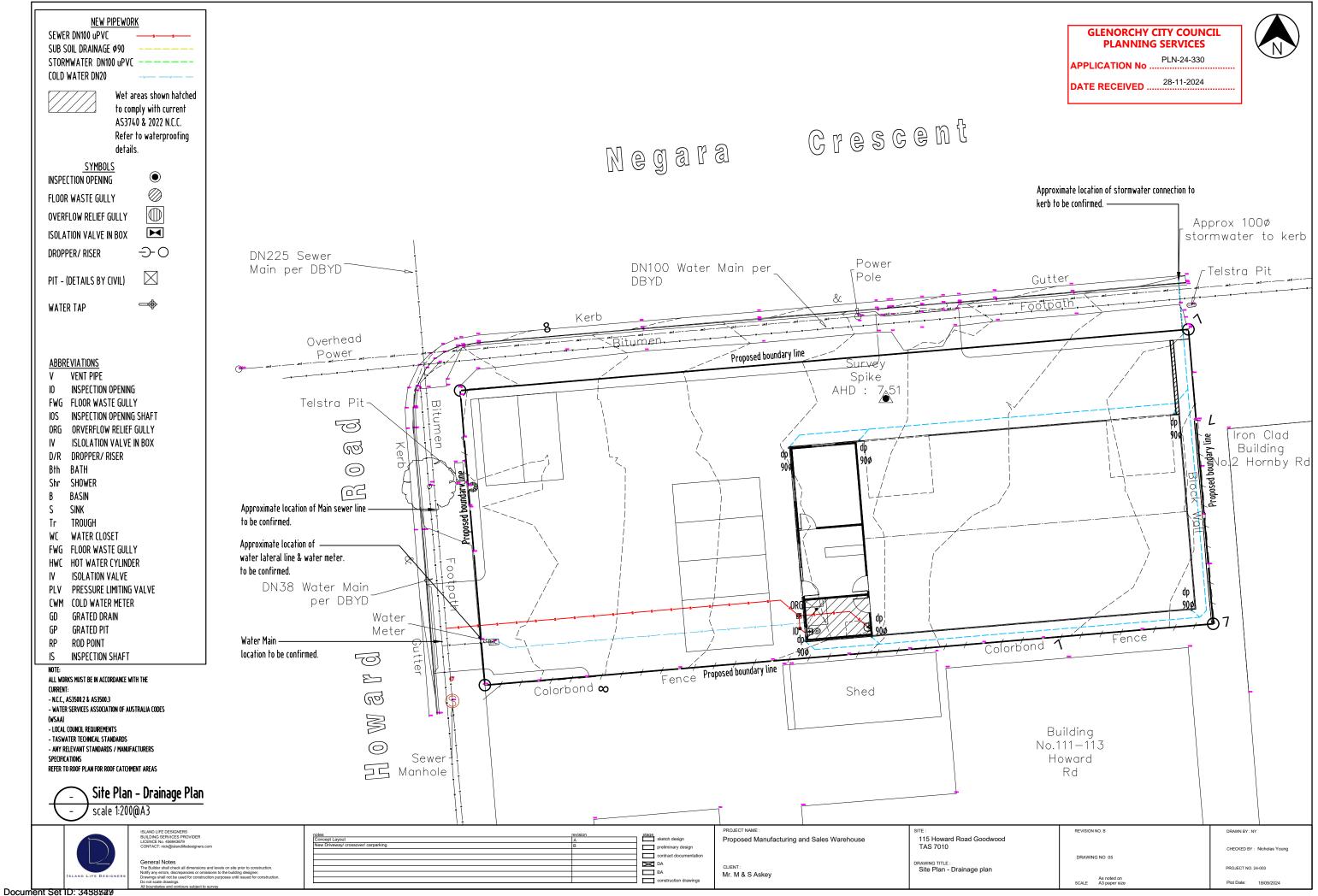
Protection Work (Section 121 of the Building Act) If excavation is to a level below that of the adjoining owner's footings, along the title boundary or within 3 metres of a building belonging to an adjoining owner, the builder must (as a miniumum) provide and maintain a guard to supervise the excavation. Adjoining owner to be notified using Form 6 (Building and Protection Work Notice) by the Building Surveyor.

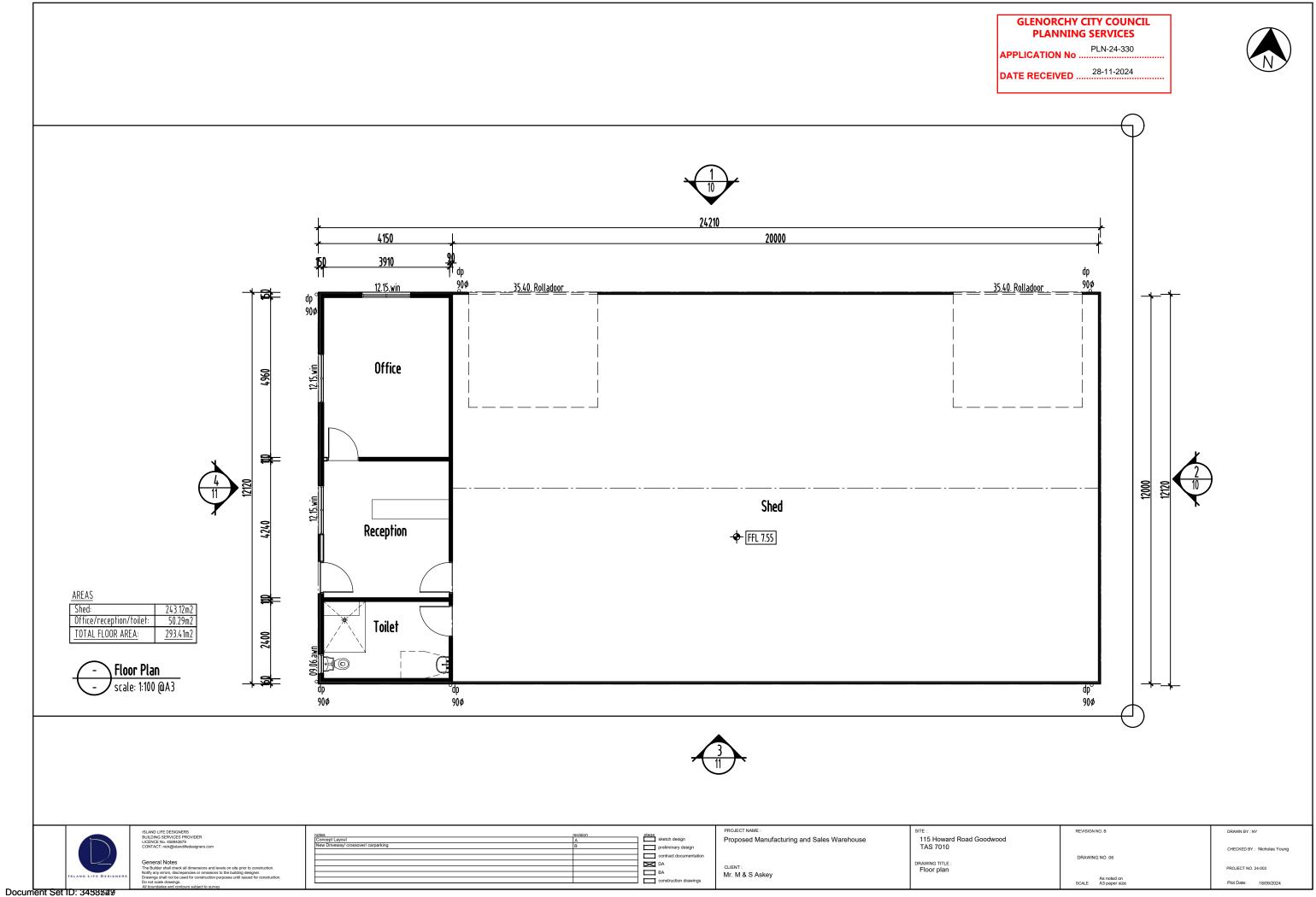


Document Set ID: 3458529 Version: 2, Version Date: 28/01/2028

Site Plan











	ISLAND LIFE DESIGNERS BUILDING SERVICES PROVIDER LICENCE NG. 458494879 CONTACT: nick@siandifiedesigners.com	notes Concept Layout New Driveway/ crossover/ carparking	revision A B	sketch design preliminary design contract documentation	PROJECT NAME : Proposed Manufacturing and Sales Warehouse	SITE : 115 Howard Road Goodwood TAS 7010
SLAND LIFE DESIGNERS	General Notes The Buidra sub-tack all dimensions and levels on site prior to construction. Notly any errors, disregancies or comsistence to the building designer. Drawings shall not be used for construction purposes until issued for construction. Do not case drawings. All boundaries and contours subject to survey			DA DA BA construction drawings	CLIENT : Mr. M & S Askey	DRAWING TITLE : Roof plan

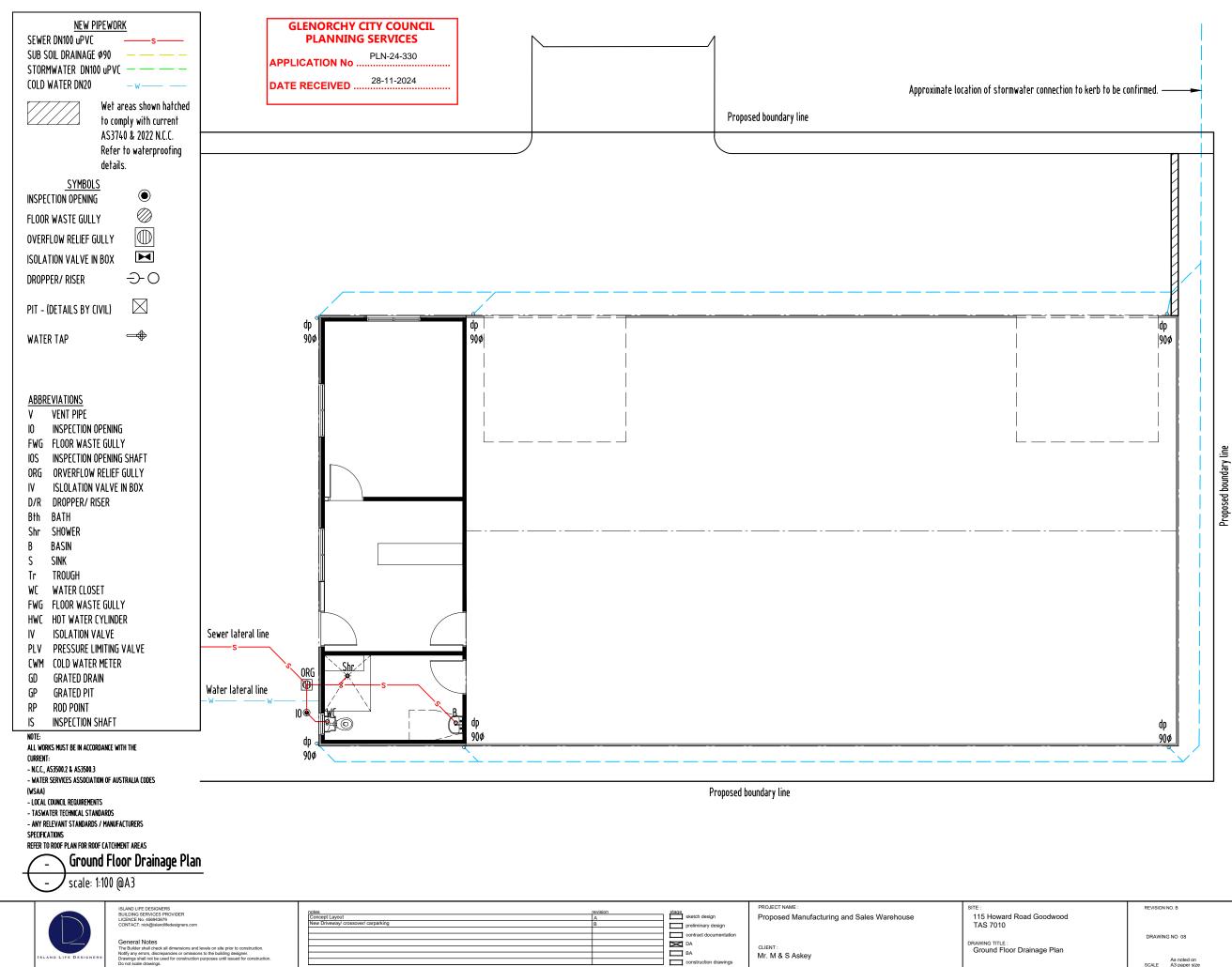
Document Set ID: 3458529 Version: 2, Version Date: 28/01/2026





28-11-2024 DATE RECEIVED

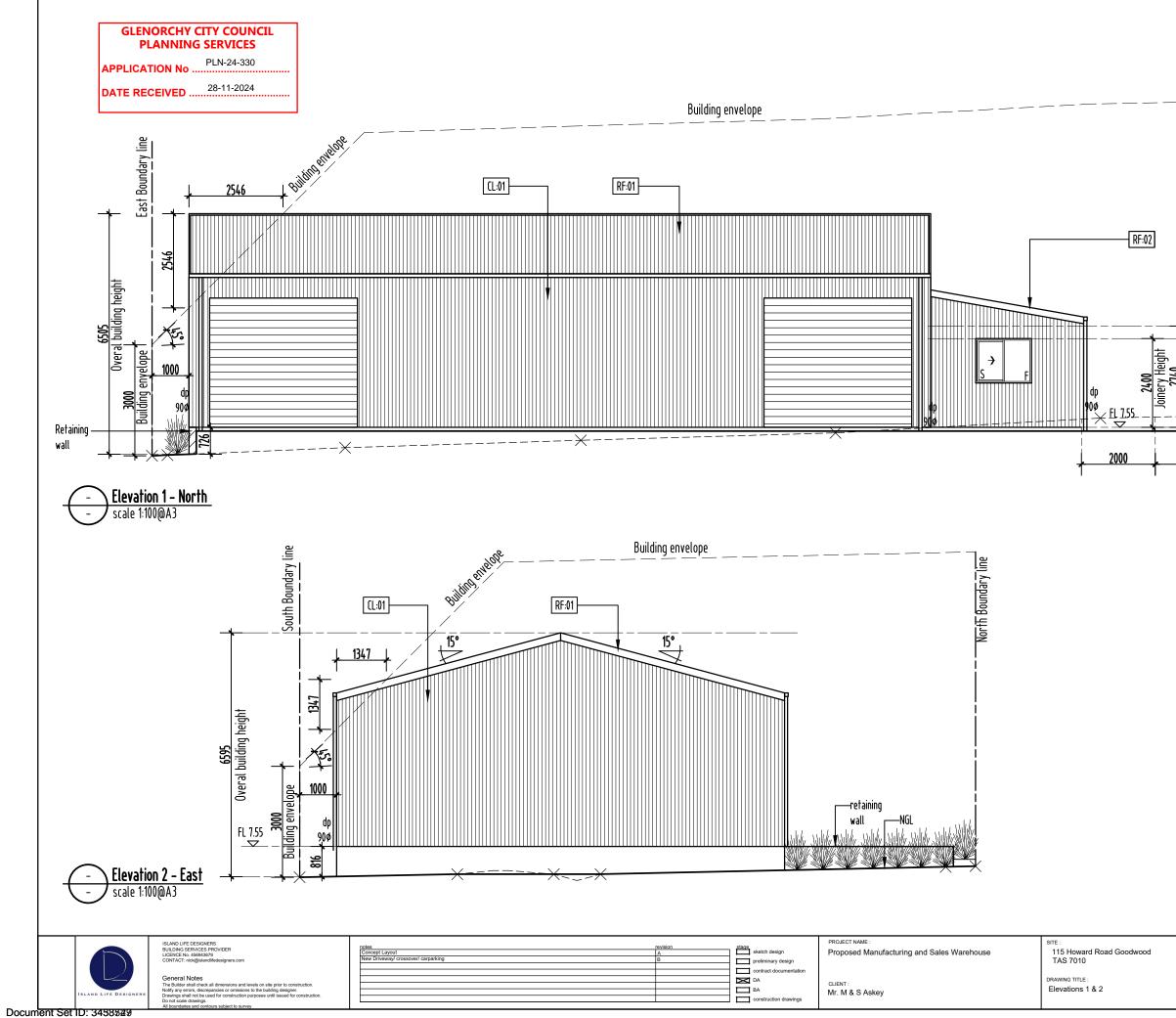
REVISION NO. B DRAWN BY : NY CHECKED BY : Nicholas Young DRAWING NO 07 PROJECT NO. 24-003 As noted on SCALE A3 paper size Plot Date: 18/09/2024



Document Set ID: 3458529 Version: 2, Version Date: 28/01/2025

REVISION NO. B	DRAWN BY : NY
DRAWING NO 08	CHECKED BY : Nicholas Young
	PROJECT NO. 24-003
As noted on SCALE A3 paper size	Plot Date: 18/09/2024



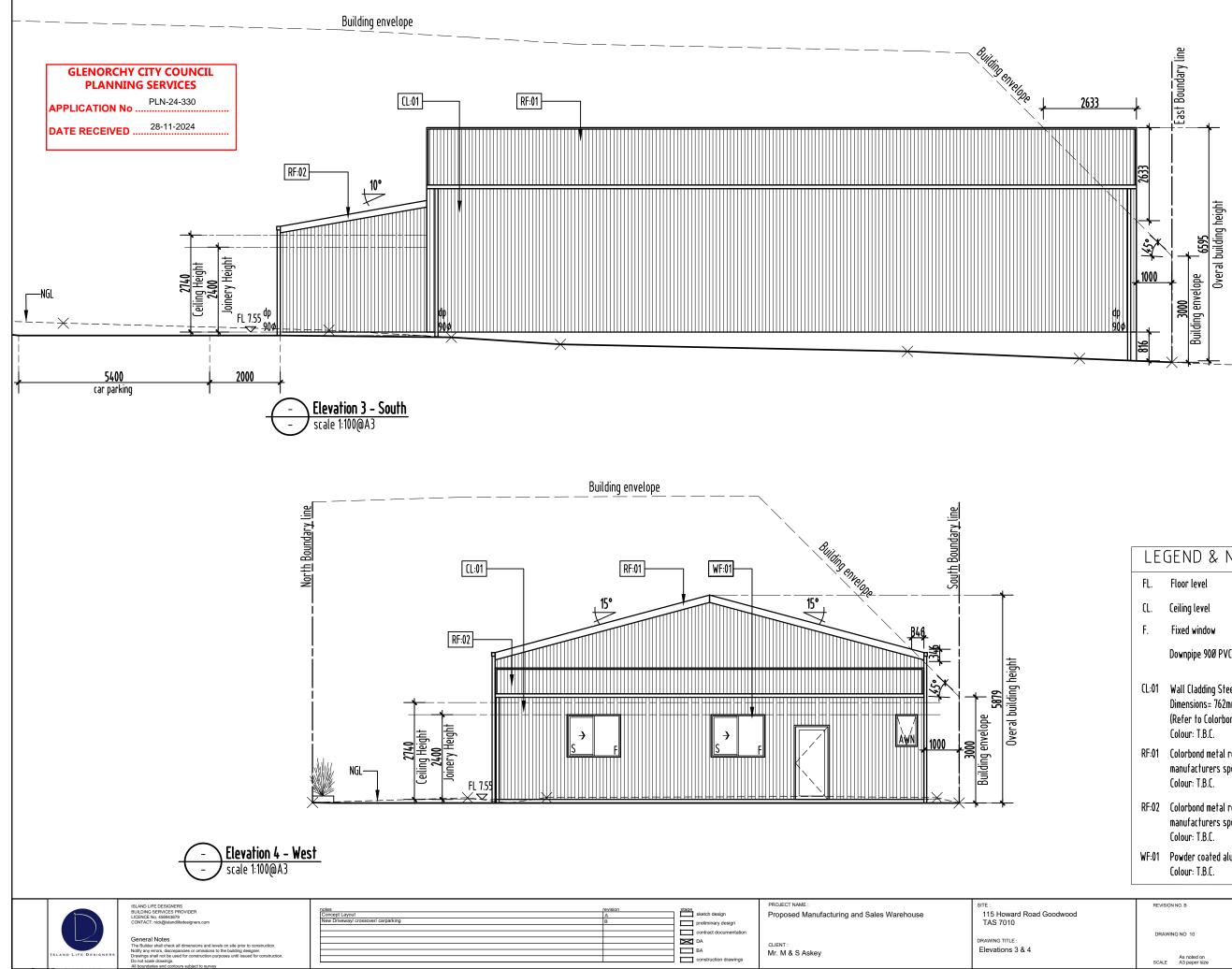


	LE	GEND & NOTE	S]
-	FL.	Floor level S	5. Slidii	ng window	
	CL.	Ceiling level A	WN. Awi	ning window	
	F.	Fixed window			
		Downpipe 900 PVC			
	(L:01	Wall Cladding Steel Clad .4. Dimensions= 762mmW thick: (Refer to Colorbond for ins Colour: T.B.C.	ness = 0.4	2mm	
	RF:01	Colorbond metal roofing 15 manufacturers specificatio Colour: T.B.C.		pe, installed as per	
	RF:02	Colorbond metal roofing 10 manufacturers specificatio Colour: T.B.C.		pe, installed as per	
	WF:01	Powder coated aluminium w Colour: T.B.C.	indow / do	or frames	
Ceiling Height				NGL	
Ceilin					
<u> </u>					
	I	5400			
		parking	┦		
	REVISI	ON NO. B		DRAWN BY : NY	
		VING NO 09		CHECKED BY : Nicholas Young	

PROJECT NO. 24-003

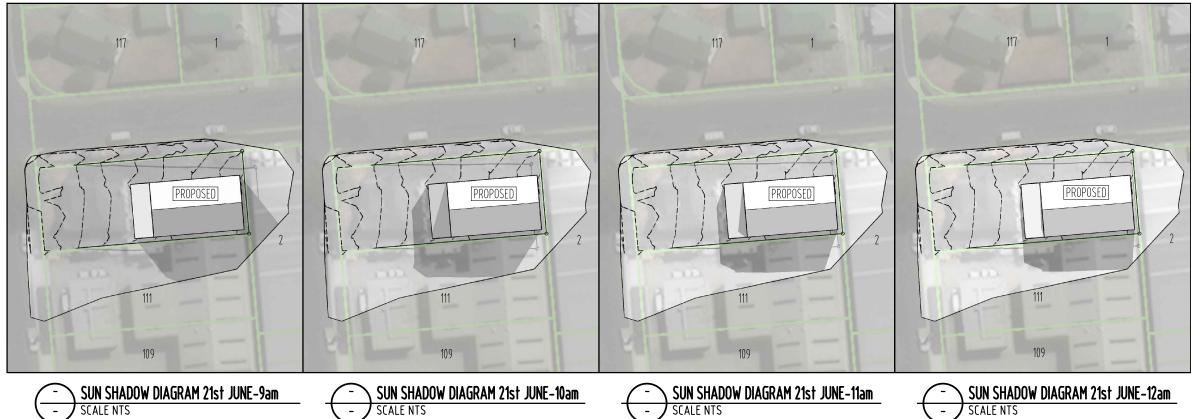
Plot Date: 18/09/2024

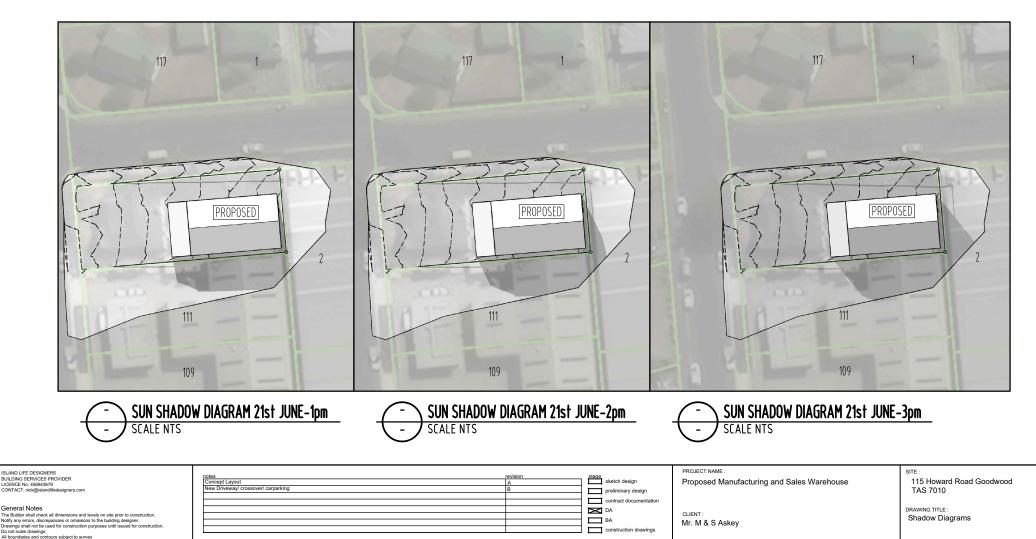
As noted on SCALE A3 paper size



FL.	GEND & NO	S.	Slidin	g window
ί.	Ceiling level	AWN.	∆wn	- ing window
F.	Fixed window			
	Downpipe 900 PVC			
CL:01	Wall Cladding Steel Cl. Dimensions= 762mmW (Refer to Colorbond fo Colour: T.B.C.	thickness	= 0.42	
RF:01	Colorbond metal roofi manufacturers specifi Colour: T.B.C.	-	f slope	e, installed as per
RF:02	Colorbond metal roofi manufacturers specifi Colour: T.B.C.	-	f slope	e, installed as per
WF:01	Powder coated alumini Colour: T.B.C.	um windov	v / doc	or frames
REVI	SION NO. B			DRAWN BY : NY
DRA	WING NO 10			CHECKED BY : Nicholas Young

PROJECT NO. 24-003 Plot Date: 18/09/2024







REVISION NO. B	DRAWN BY : NY
DRAWING NO 11	CHECKED BY : Nicholas Young
	PROJECT NO. 24-003
As noted on SCALE A3 paper size	Plot Date: 18/09/2024

CIVIL / HYDRAULIC DRAWINGS PROPOSED DRIVEWAY (NEW WAREHOUSE) MR M & S ASKEY 115 HOWARD ROAD, GOODWOOD TAS 7010

DRAWING SCHEDULE

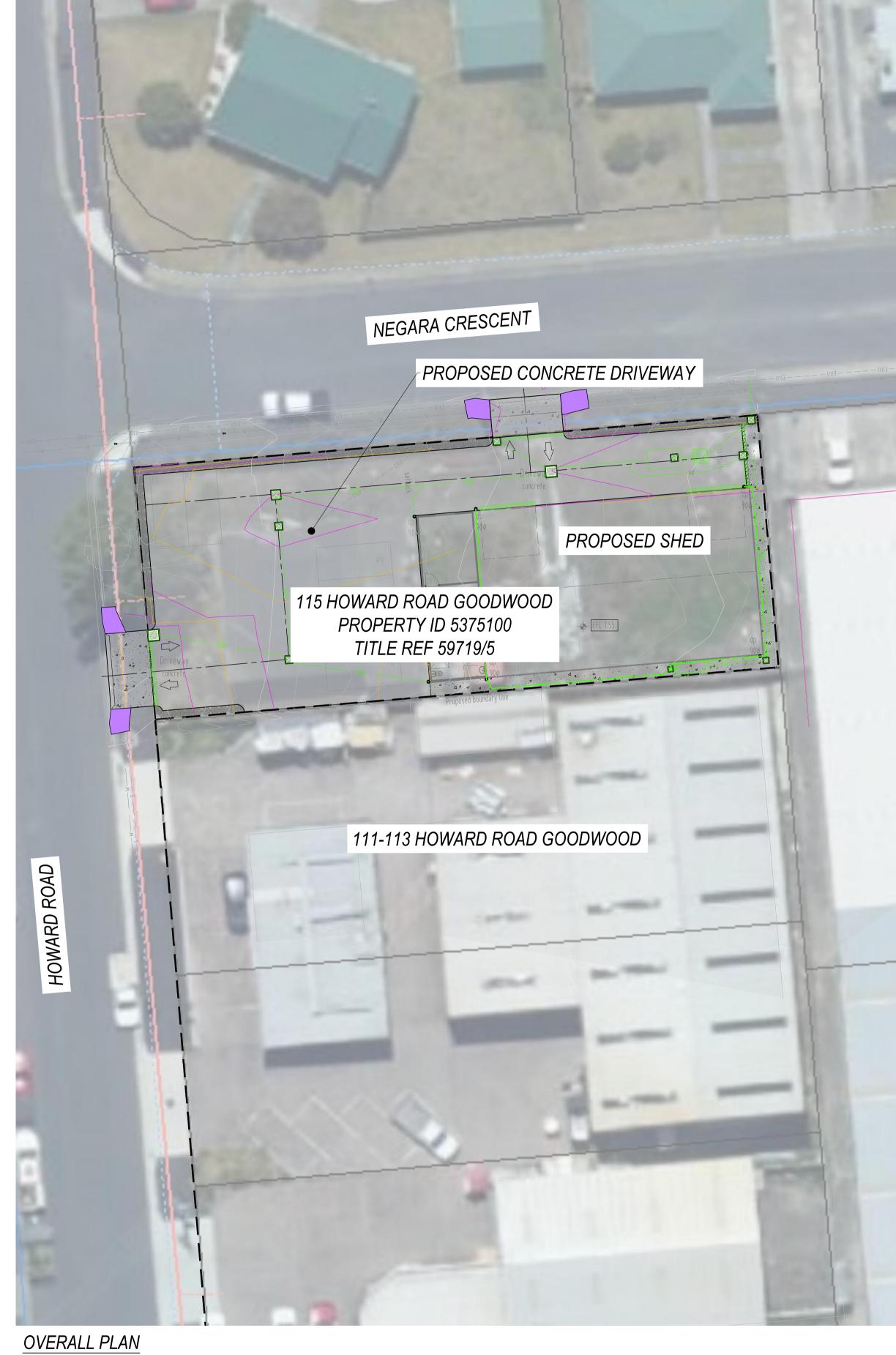
SHEET CO1	DRAWING TITLE TITLE & OVERALL PLAN	REV 3	DATE 15/01/2025
C02	NOTES & LEGEND	1	18/12/2024
C03	CIVIL OVERALL LAYOUT	1	18/12/2024
C04	CIVIL DETAILED LAYOUT 1	1	18/12/2024
C05	CIVIL DETAILED LAYOUT 2	3	15/01/2025
C06	PARKING LAYOUT	1	18/12/2024
C07	LONG SECTION	1	18/12/2024
C08	CROSS SECTIONS	1	18/12/2024
C09	CONSTRUCTION DETAILS 1	1	18/12/2024
C10	CONSTRUCTION DETAILS 2	0	15/01/2025



<u>WARNING</u> BEWARE OF UNDERGROUND SERVICES THE LOCATION OF UNDERGROUND SERVICES ARE APPROXIMATE ONLY AND THE EXACT POSITION SHOULD BE PROVEN ON SITE. NO GUARANTEE IS GIVEN THAT ALL SERVICES ARE SHOWN.

NOT FOR CONSTRUCTION

REV	DESCRIPTION		DATE	REV	DESCRIPTION	DA
0	FOR DEVELOPMENT APPROVAL	CF	14/10/2024			
1	FOR DEVELOPMENT APPROVAL	CF	18/12/2024			
2	FOR DEVELOPMENT APPROVAL	CF	09/01/2025			
3	FOR DEVELOPMENT APPROVAL	CF	15/01/2025			



SCALE 1:200 (mm) (A1)

BASE SURVEY SUPPLIED BY LEARY AND COX SURVEYED ON: 13/05/2022 HORIZONTAL DATUM: MGA2020 AHD 83 GRID: GDA2020, ZONE 55 LEVEL DATUM: AHD



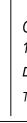
UNIT 4, 160 BUNGANA WAY CAMBRIDGE TAS PH: 0414 149 394

ACCREDITATION: BSD LICENCE NO. 479819732

FYSH DESIGN

NORTH \bigwedge K d







PROPOSED DRIVEWAY (NEW WAREHOUSE) CLIENT: MR M & S ASKEY 115 HOWARD ROAD, GOODWOOD TAS 7010

DRAWING TITLE TITLE AND OVERALL PLAN SCALE 1:100 DESIGNED CF PROJECT CKD-CIV-127

DRAWN CF SHEET NO. C01

SCALE

1:100 @ A1

REVISION - 3

LEGEND

NEW STORMWATER LINE(DN100 DWV SN6 @ MIN 1.0% GRADE NEW DOMESTIC WATER NEW DN100 DWV SN6 SEWER @ MIN 1.65% GRADE NEW DN100 CHARGED STORMWATER LINE @ MIN 1.0% GRADE SHAPED TABLE DRAIN **BOUNDARY LINE** EXISTING FENCE LINE EXISTING OVERHEAD POWER LINE EXISTING TELECOMMUNICATIONS LINE EXISTING POWER POLE

NEW STORMWATER/SEWER MANHOLE

WATER VALVE WATER METER

DOCUMENTED FORM;

SAFETY OFFICERS.

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DEPTH TO INVERT

OF OUTLET

>600 ≤900

>900 ≤1200

>1200

≤600

THE TYPE OF WORK.

IDENTIFIED HAZARDS

CONTAMINATED SOILS

CONFINED SPACES

OVERHEAD POWER LINES

WORKING AT HEIGHTS

TRAFFIC MANAGEMENT

GENERAL NOTES

- ALL PRIVATE PLUMBING WORKS SHALL GENERALLY BE IN ACCORDANCE WITH THE AS3500, NATIONAL CONSTRUCTION CODE VOL 3 (PLUMBING CODE OF AUSTRALIA), & THE IPWEA MUNICIPAL STANDARD SPECIFICATION AND DRAWINGS AS APPLICABLE. 2. UNLESS NOTED OTHERWISE THE CONTRACTOR IS REQUIRED TO OBTAIN ALL NECESSARY PERMITS FOR THE WORKS INCLUDING ANY WORKS IN THE ROAD RESERVATION AND ON ADJACENT PRIVATE PROPERTIES.
- 3. THE CONTRACTOR SHALL CONFIRM THE PRESENCE & LOCATION OF ALL EXISTING SERVICES ON THE SITE & WITHIN THE AREA OF WORKS & CLEARLY IDENTIFY ALL DANGEROUS SERVICES UNDERGROUND & OVERHEAD.
- 4. ALL DRAIN AND SERVICES TIE IN LEVELS & LOCATIONS ARE TO BE CONFIRMED BEFORE COMMENCEMENT OF CONSTRUCTION WORK. 5. UNLESS NOTED OTHERWISE ALL SERVICE CONNECTIONS TO COUNCIL OR WATER AUTHORITY SERVICE SHALL BE UNDERTAKEN BY THE COUNCIL OR
- WATER AUTHORITY AT THE CONTRACTOR'S COST. 6. ALL REDUNDANT SERVICE LINES SHALL BE CUT AND PLUGGED AT EXTERNAL BOUNDARIES. WITHIN THE SITE BOUNDARY ALL REDUNDANT SERVICES
- SHALL BE REMOVED AND DISPOSED OF. 7. REDUNDANT SERVICE TRENCHES SHALL BE BACKFILLED WITH FULLY COMPACTED MATERIAL APPROPRIATE FOR THE AREA OF THE DEVELOPMENT SITE.
- 8. ALL UNDERGROUND WATER AND SEWER WORKS MUST BE TESTED
- AND INSPECTED BY COUNCIL OR TASWATER PRIOR TO BACKFILL. 9. ALL PIPES UNDER TRAFFIC ABLE AREAS ARE TO BE BACK FILLED FULL DEPTH WITH 20MM F.C.R. AND FULLY COMPACTED.

SERVICES NOTES:

WATER SUPLY 1. ALL WATER WORKS IN PUBLIC AREAS ARE TO BE IN ACCORDANCE WITH WATER SUPPLY CODE WSA 03-2011-3.1 MRWA ED 2 AND

- TASWATER'S SUPPLEMENT. 2. ALL WATER SUPPLY WORKS IN PRIVATE AREAS SHALL BE IN ACCORDANCE WITH IN ACCORDANCE WITH WITH AS3500.1 & AS3500.4 3. ALL INTERNAL WATER SUPPLY SERVICES SHALL BE PLANNED AND INSTALLED BY THE PLUMBING CONTRACTOR IN ACCORDANCE WITH AS3500.
- 4. ALL HOT WATER LINES ARE TO BE FULLY LAGGED.
- 5. ALL HOT WATER SERVICES TO BE INSTALLED WITH TEMPERING DEVICES PROVIDING WATER AT NO GREATER THAN 45 DEGREES C. IN
- ACCORDANCE WITH THE REQUIREMENTS OF AS 3500.4. 6. ALL MODIFICATIONS AND ADDITIONS TO WATER SERVICES THAT CONNECT DIRECTLY ONTO TASWATER MAINS MUST BE CARRIED BY TASWATER AT THE CONTRACTOR'S COST.
- 7. ALL WATER SUPPLY PIPES ARE TO BE LOCATED WITH MINIMUM CLEARANCES TO OTHER SERVICES IN ACCORDANCE WITH THAT SPECIFIED IN THE WATER SUPPLY CODE WSA 03-2011-3.1 MRWA ED E - TABLE 5.5.

SERVICES NOTES:

- 1. ALL SEWER WORKS IN PUBLIC AREAS ARE TO BE IN ACCORDANCE WITH WSA 02-2002-2.3 MRWA EDITION 1.0 AND TASWATER'S SUPPLEMENT.
- 2. ALL SEWER WORKS IN PRIVATE AREAS SHALL BE IN ACCORDANCE WITH AS3500.2. 3.UNLESS NOTED OTHERWISE ALL SEWER DRAINS SHALL BE PVC SEWER CLASS "SN8" TO AS1260.
- 4.ALL SEWER MANHOLE LIDS TO BE GATIC TYPE, HEAVY DUTY FOR TRAFFIC AREAS, LIGHT DUTY FOR NON TRAFFIC AREAS.
- 5.WHERE NECESSARY ALL EXISTING MANHOLE & PIT TOPS SHALL BE ADJUSTED TO SUIT NEW SURFACE LEVELS. PROVIDE AND INSTALL NEW APPROVED LIDS WHERE NECESSARY.
- 6. PROVIDE ALL NECESSARY TESTING & INSPECTION OPENINGS TO PIPE WORK. WHERE RELEVANT PROVIDE ADDITIONAL INSPECTION OPENINGS TO ALLOW IDENTIFICATION OF THE ORIGIN OF BLOCKAGES.
- 7. ALL MAINTENANCE STRUCTURES ARE TO BE IN ACCORDANCE WITH
- WSA SEW1300 DRAWING SERIES.
- 8. NEW SEWER MAIN DRAINS SHALL BE DN150 UPVC CLASS 'SN8' TO AS 1260 U.N.O. 9. ALL PRIVATE SEWER DRAINS TO BE DN100 (UNO) PVC TO AS1260.
- 10. MANHOLES WITH INTERNAL DROPS SHALL BE 1200 INTERNAL DIAMETER MINIMUM.

NOT FOR CONSTRUCTION

FOR DEVELOPMENT APPROVAL CF 18/12/2024 0 CF 14/10/2024 FOR DEVELOPMENT APPROVAL REV DESCRIPTION DESCRIPTION DATE REV

WORKPLACE HEALTH & SAFETY NOTES:

BEFORE THE CONTRACTOR COMMENCES WORK THE CONTRACTOR SHALL UNDERTAKE A SITE SPECIFIC PROJECT PRE-START HAZARD ANALYSIS / JOB SAFETY ANALYSIS (JSA) WHICH SHALL IDENTIFY IN

HAZARDS AND RISKS TO HEALTH AND SAFETY.

• THE CONTROLS TO BE APPLIED IN ORDER ELIMINATE OR MINIMIZE THE RISK POSED BY THE

• THE MANNER IN WHICH THE RISK CONTROL MEASURES ARE TO BE IMPLEMENTED.

THESE ARE TO BE SUBMITTED TO THE SUPERINTENDENT AND/OR OTHER RELEVANT WORKPLACE

FOR THIS PROJECT; POSSIBLE HAZARDS INCLUDE (BUT ARE NOT LIMITED TO):

 EXCAVATION OF ANY TYPE & DEPTHS CONSTRUCTION IN GROUND WITH HIGH WATER TABLE

FELLING / LOPPING &/OR REMOVAL OF EXISTING TREES/VEGETATION UNDERGROUND STRUCTURES (MANHOLES / SUMPS / ETC)

UNDERGROUND STORMWATER, WATER AND SEWER PIPES TELECOMMUNICATION CABLES - BOTH UNDERGROUND & OVERHEAD ELECTRICAL/POWER CABLES - BOTH UNDERGROUND & OVERHEAD

WORKING WITH ASBESTOS CONTAINING MATERIALS

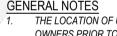
EARTHWORKS & DRIVEWAY NOTES:

ALL EARTHWORKS SHALL BE IN ACCORDANCE WITH AS3798 "GUIDELINES ON EARTHWORKS FOR COMMERCIAL AND RESIDENTIAL DEVELOPMENTS".

- ALL VEGETATION AND TOPSOIL SHALL BE STRIPPED AND GRUBBED IN THE AREA OF PROPOSED WORKS. NEW OR MODIFIED DRIVEWAY CROSSINGS SHALL BE IN ACCORDANCE WITH IPWEA STANDARD DRAWING TSD-R09-v3 AND MUST BE INSPECTED AND APPROVED BY COUNCIL.
- 4. EXCAVATED AND IMPORTED MATERIAL USED AS FILL IS TO BE APPROVED BY THE ENGINEER PRIOR TO INSTALLATION. FILL MATERIAL SHALL BE WELL GRADED AND FREE OF BOULDERS OR COBBLES EXCEEDING 150mm IN DIAMETER UNLESS
- APPROVED TO BE OTHERWISE 6. FILL REQUIRED TO SUPPORT DRIVEWAYS INCLUDING FILL IN EMBANKMENTS THAT SUPPORT DRIVEWAYS SHALL BE INSTALLED IN ACCORDANCE WITH THE FOLLOWING REQUIREMENTS:
 - TOP SOIL AND ORGANIC MATTER SHALL BE STRIPPED TO A MINIMUM OF 100mm.
 - THE SUB GRADE SHALL HAVE A MINIMUM BEARING CAPACITY OF 100 kPa.
 - FILL IN EMBANKMENTS SHALL BE KEYED 150mm INTO NATURAL GROUND. • THE FILL SHALL BE COMPACTED IN HORIZONTAL LAYERS OF NOT MORE THAN 200mm.
 - EACH LAYER SHALL BE COMPACTED TO A MINIMUM DENSITY RATIO OF 95% STD. IT IS THE BUILDERS
- RESPONSIBILITY TO ENSURE THAT THIS IS ACHIEVED. 7. WHERE THE ABOVE REQUIREMENTS CANNOT BE ACHIEVED THE ENGINEER SHALL BE CONSULTED AND THE FORMATION SHALL BE PROOF ROLLED (UNDER SUPERVISION OF THE ENGINEER) TO CONFIRM AN APPROVED BASE.
- CONCRETE PAVEMENTS SHALL BE CURED FOR A MINIMUM OF 3 DAYS USING A CURRENT BEST PRACTICE METHOD. 9. SAWN CONTROL JOINTS SHALL BE CONSTRUCTED AS SOON AS POSSIBLE WITHOUT RAVELLING THE JOINT, GENERALLY THIS SHALL BE WITHIN 24 HOURS.
- 10. BATTERS SHALL BE SET TO A SAFE ANGLE OF REPOSE IN ACCORDANCE WITH THE BCA VOL 2 AS INDICATED BELOW:

NOTE: WHERE SITE CONDITIONS ARE UNSUITABLE FOR A BATTERED BANK CONSULT THE DESIGNER OR ENGINEER FOR A SUITABLE RETAINING WALL DESIGN. EMBANKMENTS THAT ARE TO BE LEFT EXPOSED MUST BE STABILISED BY VEGETATION OR SIMILAR WORKS TO PREVENT SOIL EROSION.

SEE TA	ABLE BELOW				
		IL TYPE R BCA 3.2.4)	EMBANKMEN	T SLOPES H:L	
		R DOA 3.2.4)	COMPACTED FILL	СИТ	
24	STABLE	ROCK (A*)	2:3	8:1	
	SA	ND (A*)	1:2	1:2	
	SI	LT (P*)	1:4	1:4	1
	CLAX	FIRM CLAY	1:2	1:1	1
	CLAY	SOFT CLAY	NOT SUITABLE	2:3	
	SOFT SOILS (P)		NOT SUITABLE	NOT SUITABLE]



- OWNERS PRIOR TO COMMENCING CONSTRUCTION. ALL WORKS SHALL BE IN ACCORDANCE WITH LGAT STANDARD DRAWINGS (U.N.O.) ALL DIMENSIONS ARE IN METRES UNLESS NOTED OTHERWISE (U.N.O.)
- **CIVIL WORKS**
- THE SUPERINTENDENT.

- PAVEMENT MARKING. R80 MISCELLANEOUS CONCRETE SLABS.
- THAN REMOVAL OF TREES IDENTIFIED AS IN A HAZARDOUS CONDITION.
- SURFACE REINSTATEMENT & EROSION CONTROL.

SERVICES NOTES STORMWATER

- . ALL STORMWATER WORKS TO BE IN ACCORDANCE WITH AS3500.3. 2. ALL STORM WATER PIPES LESS THAN DN300 TO BE UPVC CLASS "SN8" TO AS 1254 UNO. 3. ALL STORMWATER PIPES DN300 & LARGER TO BE 'BLACKMAX' UNO.
- SOCK SLEEVING AND FREEE DRAINING BEDDING MATERIAL.

- (SEE ADJACENT)

MINIMUM INTERNAL DIMENSIONS mm LENGTH 450 600

WIDTH

450

600

600

900

900 900



UNIT 4, 160 BUNGANA WAY CAMBRIDGE TAS PH: 0414 149 394

FYSH DESIGN

ACCREDITATION: BSD LICENCE NO. 479819732

DATE

THE LOCATION OF UNDERGROUND SERVICES ARE INDICATIVE ONLY. THE EXACT POSITION OF EACH SERVICE PRESENT SHOULD BE ESTABLISHED ON SITE WITH THE RESPECTIVE SERVICE

1. THE CONTRACTOR SHALL PREPARE AND PROVIDE A SEDIMENT AND EROSION CONTROL PLAN FOR THE WORKS. NO WORK SHALL COMMENCE UNTIL THIS PLAN HAS BEEN APPROVED BY

NO MACHINERY IS TO BE PLACED ON OR HAVE ACCESS TO ANY AREA OUTSIDE THE LIMIT OF WORKS UNLESS APPROVED BY THE PRINCIPAL.

THE LIMIT OF WORKS LINE SHALL BE TEMPORARILY FENCED WITH BUNTING BEFORE ANY WORKS COMMENCE.

ALL WORKS TO BE UNDERTAKEN IN ACCORDANCE WITH THE FOLLOWING DEPARTMENT OF STATE GROWTH SPECIFICATIONS: R21 - CLEARING AND GRUBBING, R22 - EARTHWORKS, R23 - SUBGRADE ZONE, R31 - OPEN DRAINS AND CHANNELS, R36 - KERB AND GUTTER, R40 - PAVEMENT BASE AND SUBBASE, R40.1

NOMINATION OF MATERIALS FORM, EXPLANATORY NOTES, R43 - PAVEMENT AND SHOULDER MAINTENANCE, R51 - SPRAYED BITUMINOUS SURFACING, R55 - ASPHALT PLACEMENT, R64 -

NO CLEARING OF VEGETATION OR REMOVAL OF TOPSOIL IS PERMITTED IN ANY AREA NOT DIRECTLY RELATED TO THE CONSTRUCTION WORKS OR AS NOTED ON THE DRAWINGS OTHER

ALL STRIPPED TOPSOIL IS TO BE STORED IN AN APPROVED MANNER FOR REHABILITATION WORKS AND VEGETATION RESEEDING.

ALL DISTURBED AND BARE GROUND INCLUDING ALL CUT & FILL SURFACES SHALL BE REHABILITATED AS FOLLOWS: REPLACE TOPSOIL WITH THAT RESERVED WHEN THE SITE WAS STRIPPED (50 THICK). RE-SEED ALL DISTURBED GROUND USING SEED MIX APPROVED BY THE SUPERINTENDENT.

14/7mm TWO COAT SEAL TO BE IN ACCORDANCE WITH DEPARTMENT OF STATE GROWTH STANDARD SPECIFICATION R51 - BITUMINOUS SURFACING. SUBGRADE CBR FOR ROAD PAVEMENTS AND FOOTPATHS TO BE A MINIMUM OF 5%

ALL PAVEMENT MARKING TO BE STANDARD PAINT IN ACCORDANCE WITH DEPARTMENT OF STATE GROWTH SPECIFICATION R64 - PAVEMENT MARKING. TRAFFIC MANAGEMENT PLAN INDICATING HOW, SAFE USE MCROBIES RD WILL BE MAINTAINED DURING CONSTRUCTION SHALL BE SUBMITTED PRIOR TO COMMENCEMENT OF WORK.

CONCRETE FOOTPATH TO BE CONSTRUCTED IN ACCORDANCE WITH LGAT STANDARD DRAWINGS TSD-R11-v3.

CONCRETE KERBS TO BE CONSTRUCTED IN ACCORDANCE WITH LGAT STANDARD DRAWINGS TSD-R14-v3.

4. ALL SUBSOIL DRAINS SHALL COMPRISE DN80 CLASS 400 SN8 POLYETHYLENE PIPE TO AS2439.1 WITH PROPRIETARY POLYESTER PIPE FILTER

5. PROVIDE ANCHOR BLOCKS IN ACCORDANCE WITH MSD SD-5005 WHERE PIPE GRADES EXCEED 15 %.

6. CONNECTIONS TO LIVE COUNCIL MAINS TO BE CARRIED OUT BY COUNCIL AT DEVELOPERS COST.

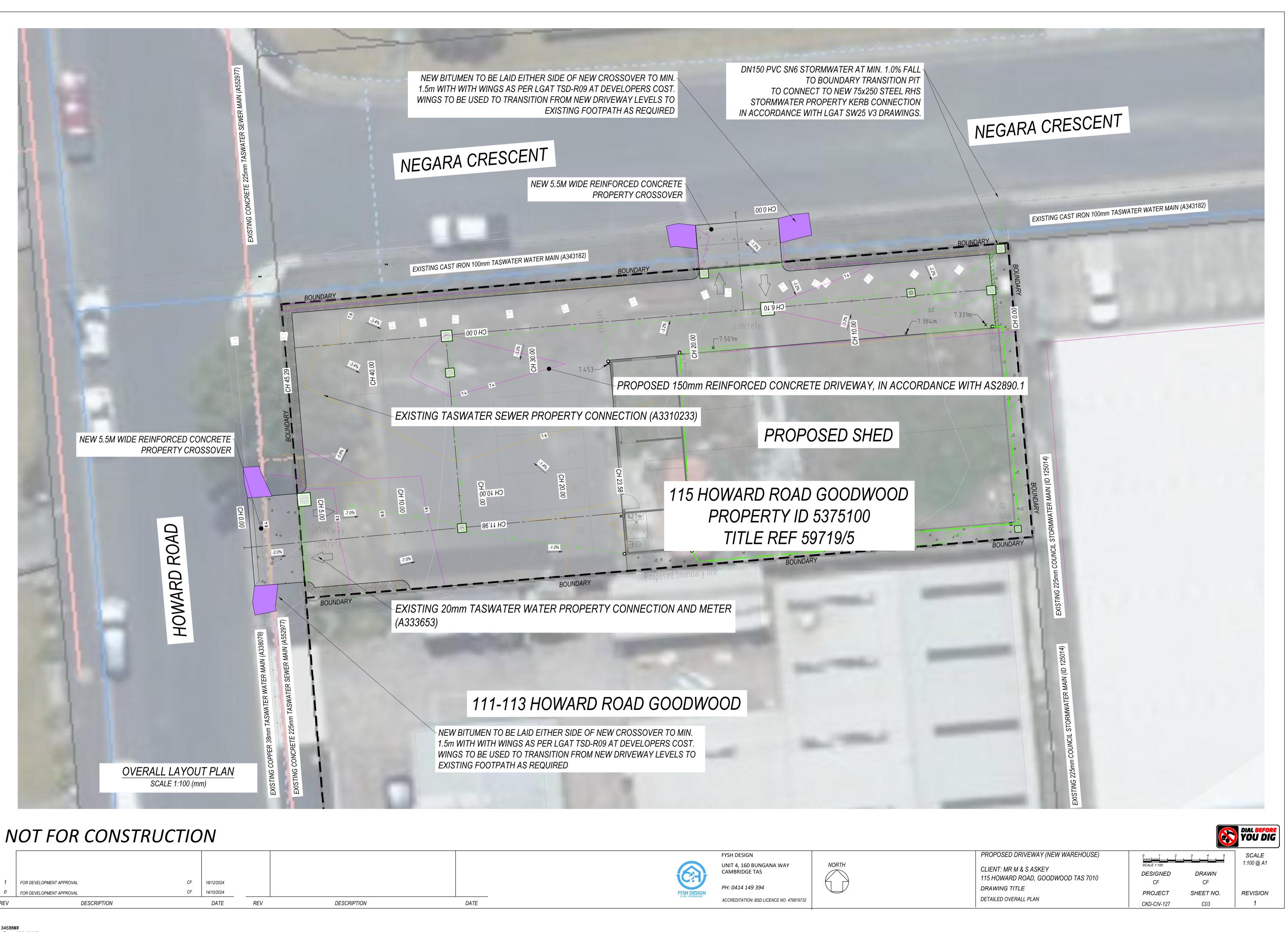
7. ALL DRAIN AND TRENCH CONSTRUCTION SHALL COMPLY WITH THE MUNICIPAL STANDARD DRG MSD SD 5001 8. ALL MANHOLE LIDS IN TRAFFICABLE AREAS SHALL COMPLY WITH CLASS "C" LOAD RATING TO AUSTRALIAN STANDARD AS 3996.

PIT DIMENSIONS SHOWN HAVE BEEN DESIGNED BY PIT CAPACITY TABLES. THESE PITS MAY NEED TO BE INCREASED IN MINIMUM INTERNAL SIZE

DUE TO THE DEPTH AS PER AS3500.3 AS PER TABLE BELOW WHICH IS THE CONTRACTORS RESPONSIBILITY TO ENSURE COMPLIANCE TO AS3500:

			YOU DIG
PROPOSED DRIVEWAY (NEW WAREHOUSE)			SCALE 1:100 @ A1
CLIENT: MR M & S ASKEY 115 HOWARD ROAD, GOODWOOD TAS 7010	SCALE 1:100 DESIGNED CF	DRAWN CF	1.100 @ AT
	PROJECT	SHEET NO.	REVISION
NOTES AND LEGEND	CKD-CIV-127	C02	1

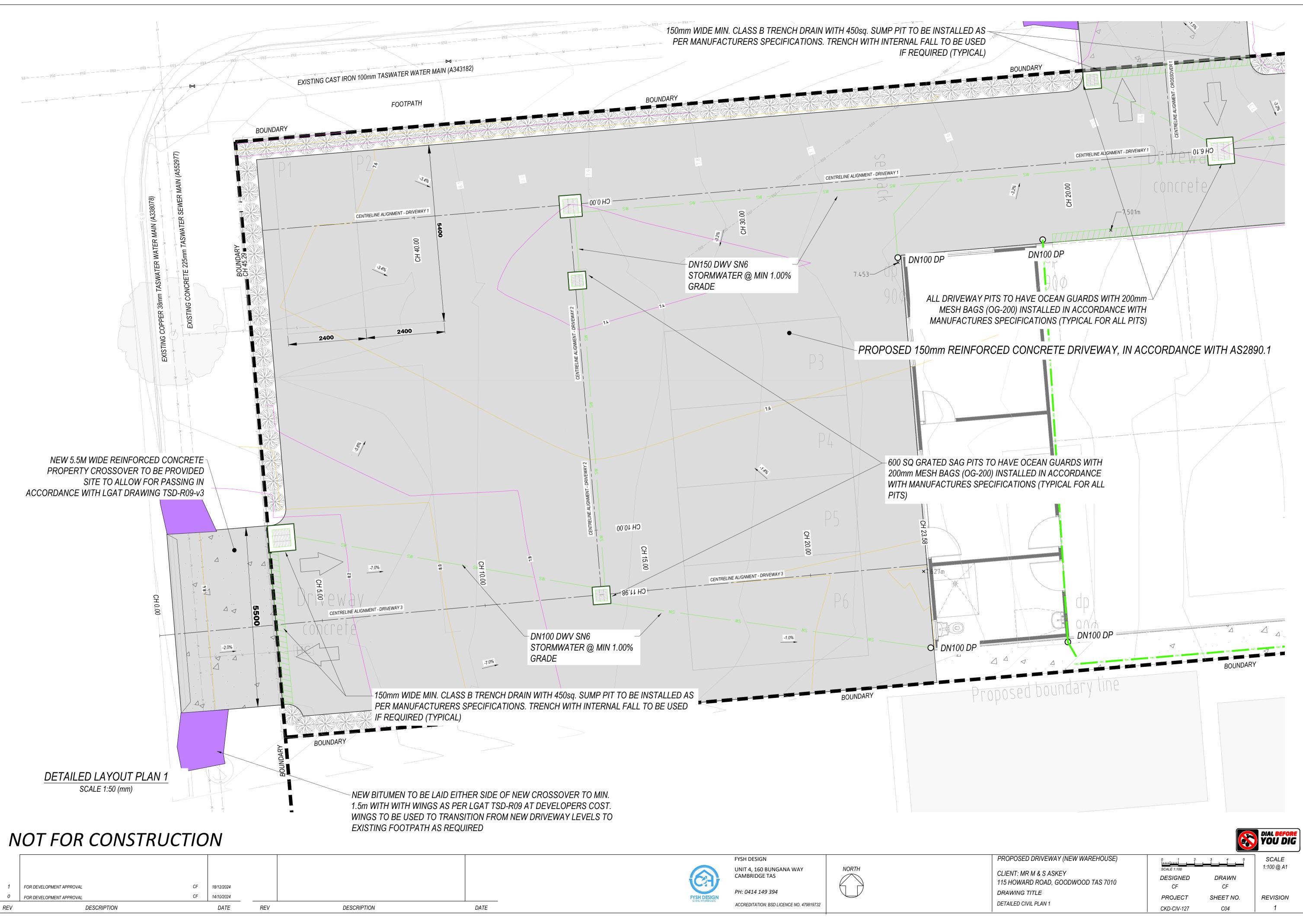
DIAL BEFORE

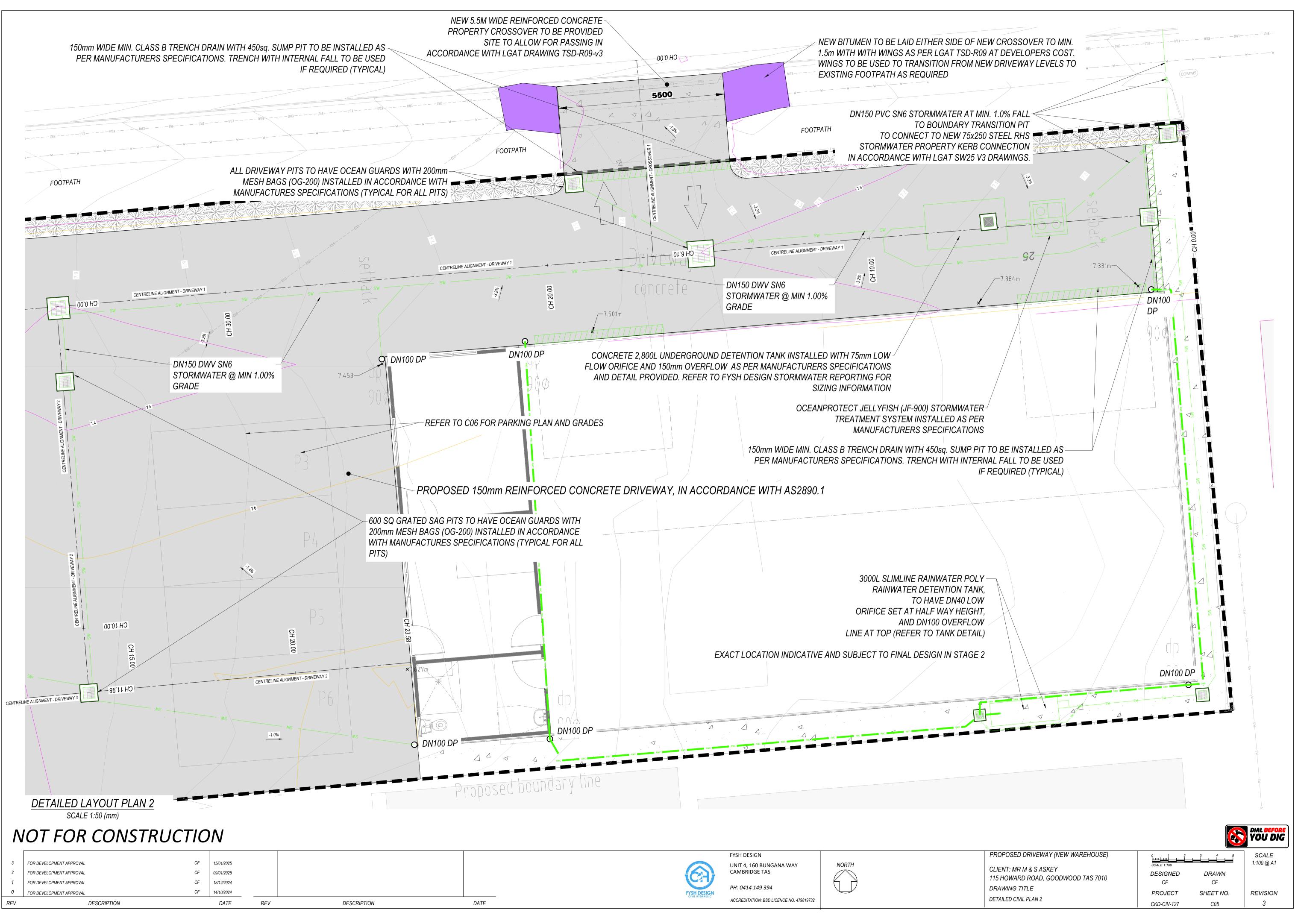


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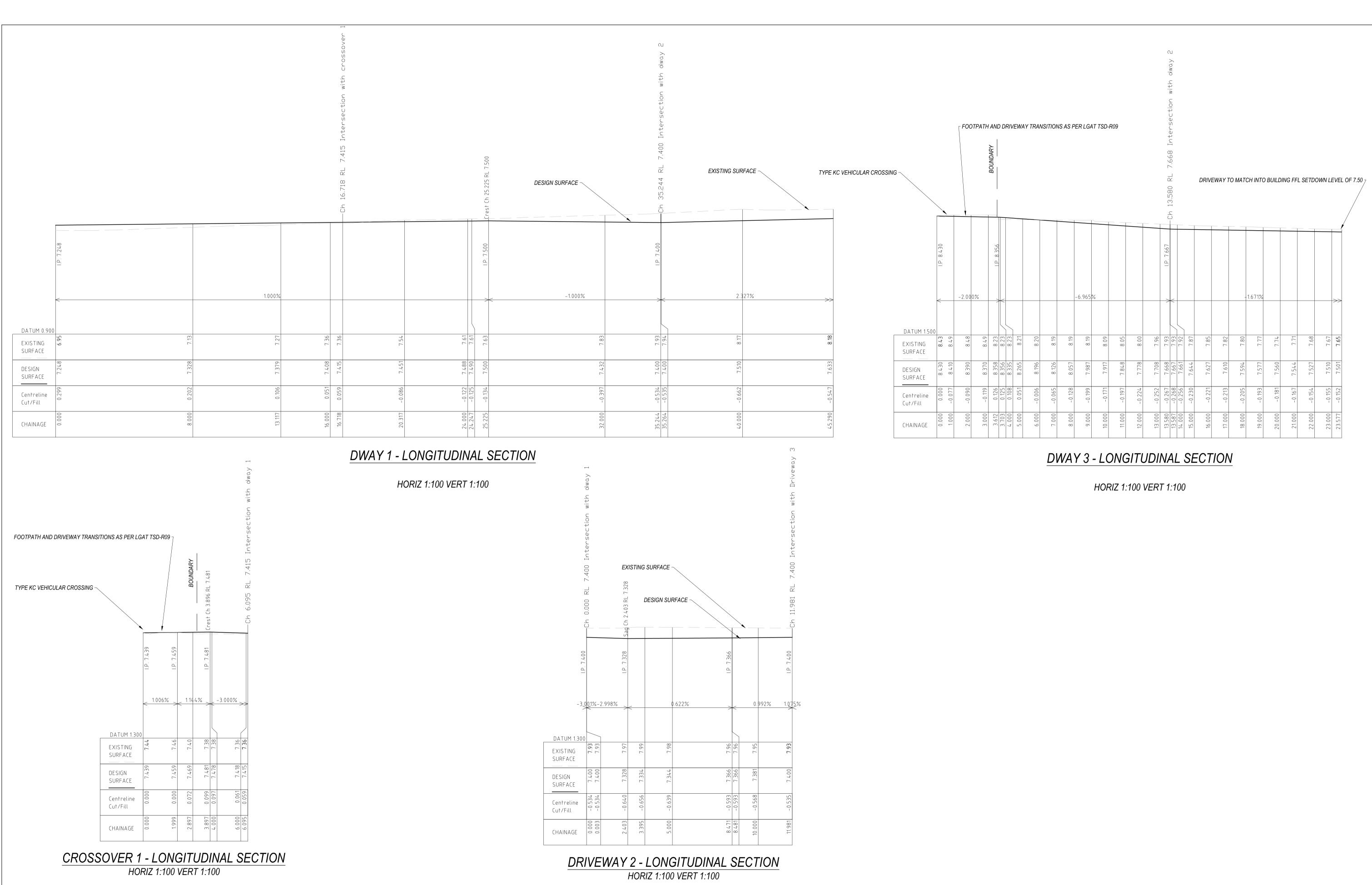








CH 20:00	Drive Wdy concrete
WITHIN PARKING AREAS BELOW 5% AS	SHOWN AND
oposed boundary line	
PROPOSED DRIVEWAY (NEW WAREHOUSE) CLIENT: MR M & S ASKEY 115 HOWARD ROAD, GOODWOOD TAS 7010 DRAWING TITLE PARKING	Impute 12345SCALESCALE 1:100DESIGNEDDRAWNSCALE1:100 @ A1DESIGNEDDRAWNCFCFREVISIONPROJECTSHEET NO.REVISION1CKD-CIV-127C061



NOT FOR CONSTRUCTION

REV	DESCRIPTION		DATE	REV	DESCRIPTION	I
0	FOR DEVELOPMENT APPROVAL	CF	14/10/2024			
1	FOR DEVELOPMENT APPROVAL	CF	18/12/2024			





PROPOSED DRIVEWAY (NEW WAREHOUSE)

CLIENT: MR M & S ASKEY 115 HOWARD ROAD, GOODWOOD TAS 7010 DRAWING TITLE LONG SECTIONS

0 1 2 3 4 5 SCALE 1:100 DESIGNED CF PROJECT CKD-CIV-127

DRAWN CF SHEET NO. C07

REVISION 1

SCALE 1:100 @ A1

RL6.1r DESIGN SURFACE EXISTING LEVEL OFFSET

RL6.3m DESIGN SURFACE EXISTING LEVEL

OFFSET

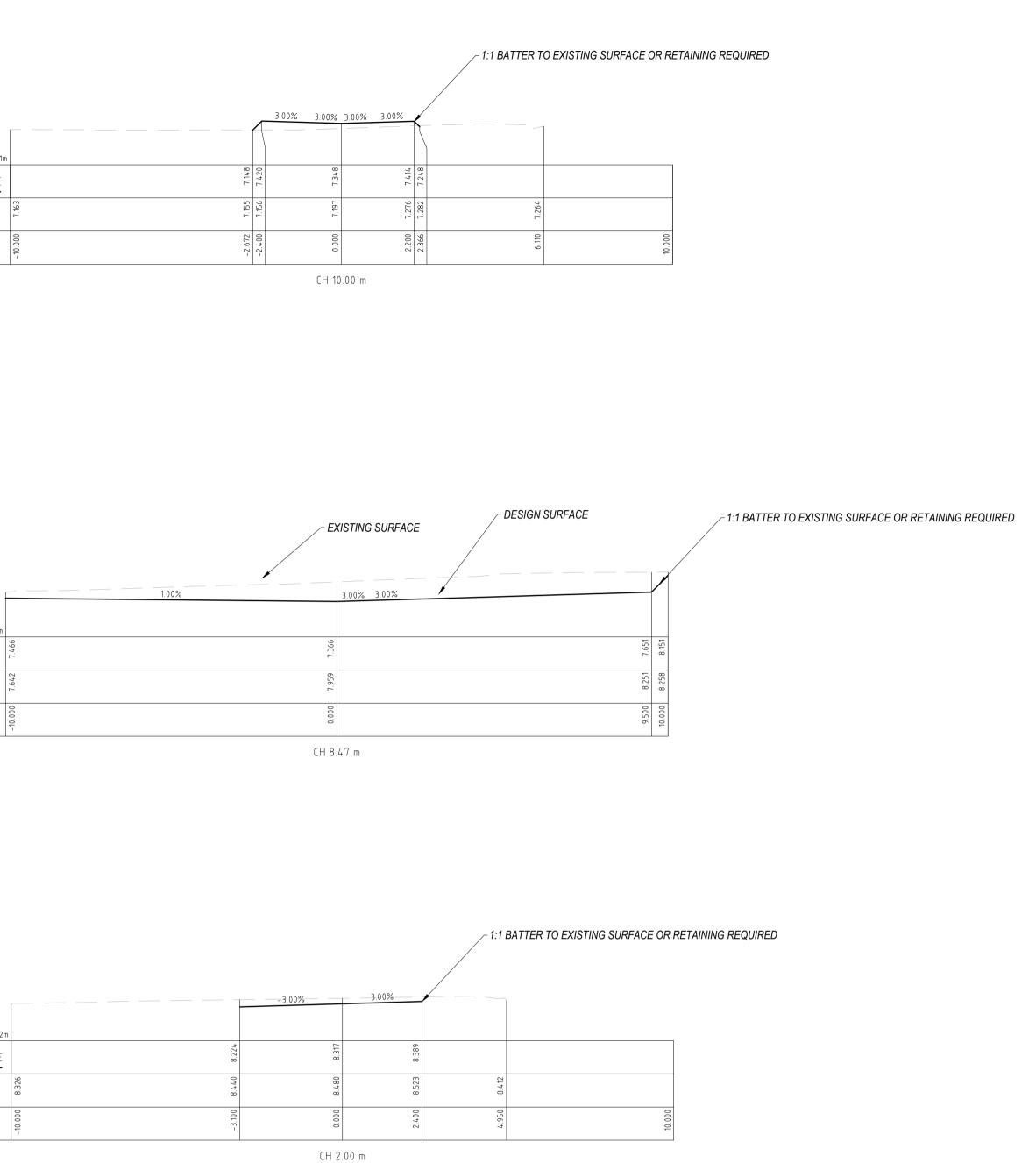
RL7.2m DESIGN SURFACE EXISTING LEVEL

OFFSET

NOT FOR CONSTRUCTION

REV	DESCRIPTION		DATE	REV	DESCRIPTION	DA
0	FOR DEVELOPMENT APPROVAL	CF	14/10/2024			
1	FOR DEVELOPMENT APPROVAL	CF	18/12/2024			

Document Set ID: 3453869 Version: 2, Version Date: 22/01/2025



DWAY - CROSS SECTIONS HORIZ 1:100 VERT 1:100



FYSH DESIGN UNIT 4, 160 BUNGANA WAY CAMBRIDGE TAS

PH: 0414 149 394 ACCREDITATION: BSD LICENCE NO. 479819732 PROPOSED DRIVEWAY (NEW WAREHOUSE)

CLIENT: MR M & S ASKEY 115 HOWARD ROAD, GOODWOOD TAS 7010 DRAWING TITLE CROSS SECTIONS

0 1 2 3 4 5 SCALE 1:100 DESIGNED CF PROJECT CKD-CIV-127

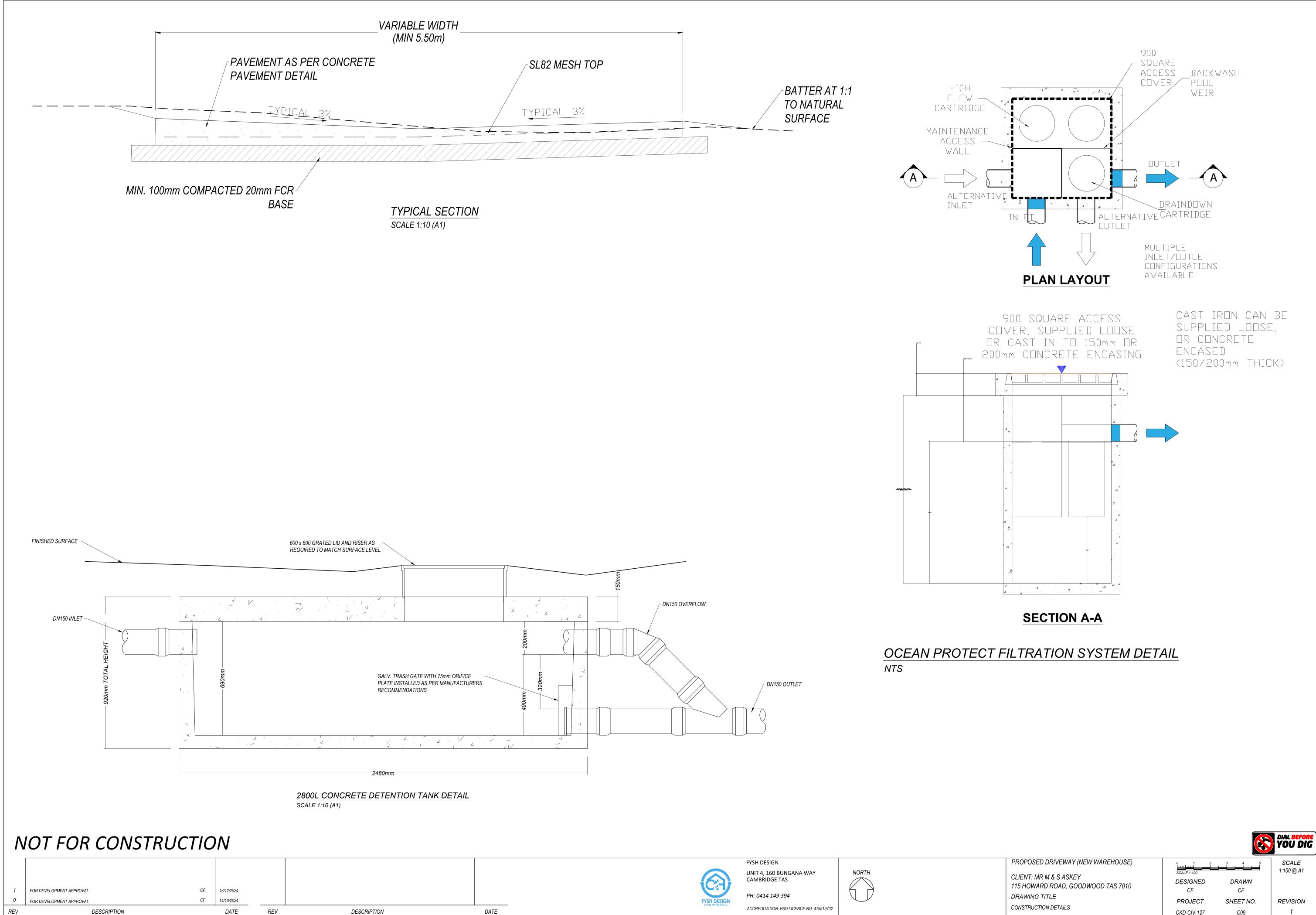
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DIAL BEFORE YOU DIG

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REVISION

1

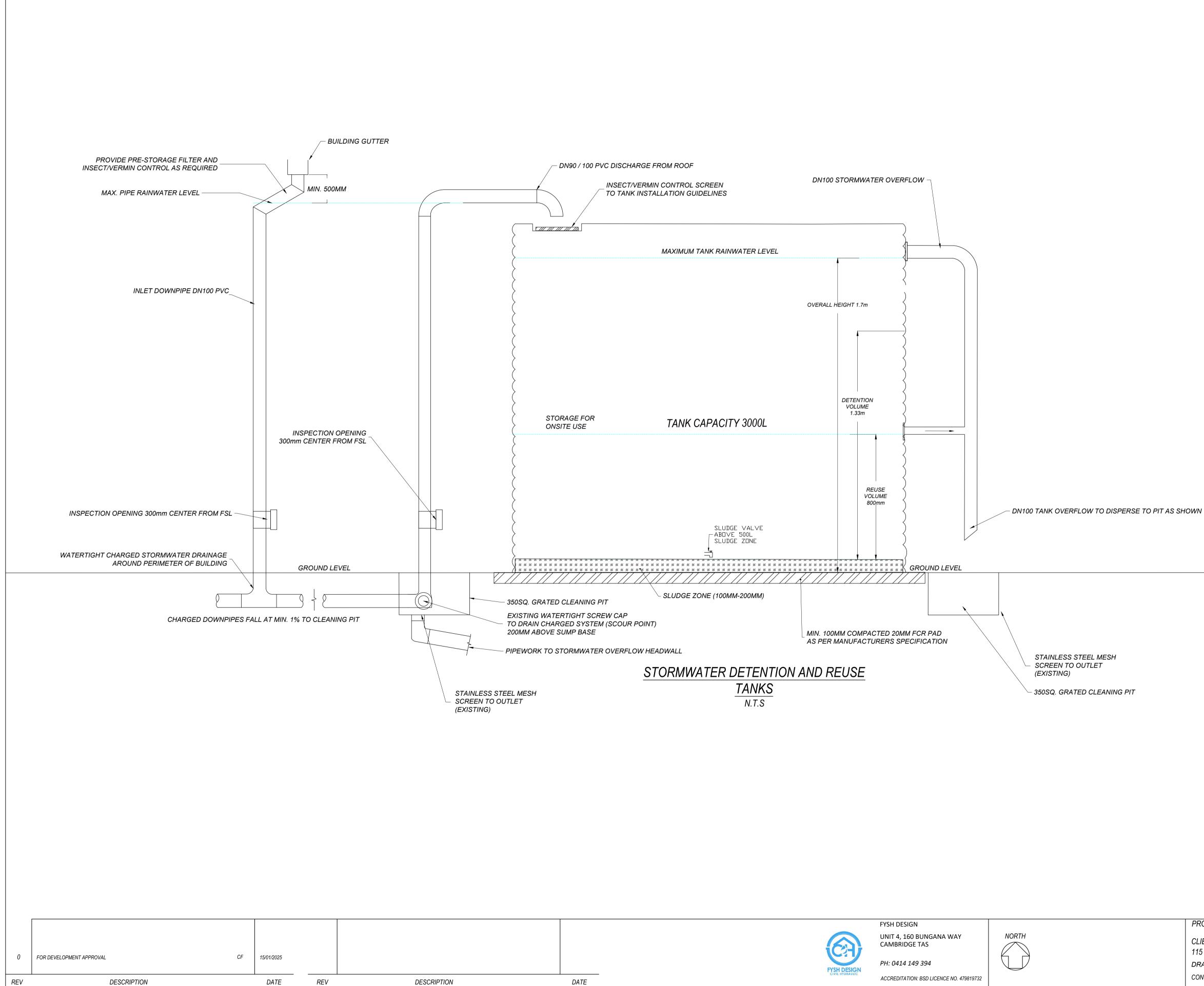




CONSTRUCTION DETAILS

CKD-CIV-127

C09





CLIENT: MR M & S ASKEY 115 HOWARD ROAD, GOODWOOD TAS 7010 DRAWING TITLE **CONSTRUCTION DETAILS 2**

DESIGNED CF PROJECT CKD-CIV-127

DRAWN CF SHEET NO. C10

DIAL BEFORE YOU DIG

SCALE 1:100 @ A1

REVISION

0

PROPOSED DRIVEWAY (NEW WAREHOUSE)

STORMWATER DESIGN REPORT

Mr M & S Askey Island Life Designers

115 Howard Road, Goodwood TAS

CF Design Reference: CKD-CIV-127

Date:15/01/2025

For DA Approval Rev 3

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- 3. DRAINAGE LAYOUT 3A. PIPE SIZING
- 4. TREATMENT
- 5. MAINTENANCE
- 6. CONCLUSION

1. INTRODUCTION AND SCOPE OF ENGAGEMENT

Fysh Design have been engaged to design a detention stormwater system for the proposed light commercial development at 115 Howard Road Goodwood. As a condition of the Glenorchy City Council Stormwater Policy, for a development adding an additional impervious area greater than 250 square meters, engineering calculations must be provided for a stormwater detention system to ensure the sites post-development peak discharge must does exceed previous existing run-off for the 5% AEP peak storm event. The following report outlines the methodology and assumptions used to ensure the proposed development complies with the Glenorchy City Council Stormwater Policy.

The site itself currently includes one single dwelling, which discharges to a stormwater kerb connection. This existing dwelling will be demolished. The proposed development involves the addition of a new large-scale shed and an associated driveway at the front of the

property. The site itself slopes gently at approximately 5-8% away from the street frontage. It is the intention of Fysh Design to upgrade the stormwater kerb connections capacity and upgrade the connection in line with the latest standards.

2. DETENTION MODEL

Rainfall depths used for the analysis are as follows (ARR DATA HUB). Multiple durations of the 5% AEP event were simulated to determine the critical storm duration for the post-development scenario:

TABLE 1: IFD DEPTHS

Design Storm Event	Design Rainfall (mm/hr)
5% AEP 5 min	84.9
5% AEP 10 min	63.9
5% AEP 20 min	44.1
5% AEP 30 min	34.7
5% AEP 60 min	22.7
5% AEP 120 min	15.1

CIVIL HYDRAULIC

Table Char	t					Uni	t: mm/h
Annual Exceedance Probability (AEP)							
Duration	63.2%	50%#	20%*	10%	5%	2%	1%
1 <u>min</u>	60.7	68.9	96.5	117	138	169	194
2 <u>min</u>	52.3	58.9	80.2	95.1	110	129	143
3 <u>min</u>	46.2	52.2	71.5	85.2	99.1	117	131
4 <u>min</u>	41.6	47.1	65.1	78.0	91.2	109	123
5 <u>min</u>	38.0	43.1	59.9	72.2	84.8	<mark>1</mark> 02	117
10 <u>min</u>	27.5	31.3	44.1	53.7	63.8	<mark>7</mark> 8.5	90.8
15 <u>min</u>	22.3	25.3	35.7	43.6	51.8	63.9	74.1
20 <u>min</u>	19.1	21.7	30.5	37.1	44.1	54.3	62.8
25 <u>min</u>	16.9	19.2	26.9	32.7	38.7	47.4	54.7
30 <u>min</u>	15.3	17.3	24.2	29.3	34.7	42.3	48.7
45 <u>min</u>	12.2	13.8	19.2	23.1	27.1	32.7	37.2
1 hour	10.4	11.8	16.2	19.5	22.7	27.2	30.7
1.5 hour	8.32	9.42	12.9	15.4	17.8	21.1	23.6
2 hour	7.12	8.06	11.0	13.1	15.1	17.7	19.7
3 hour	5.72	6.49	8.89	10.5	12.0	14.1	15.6
4.5 hour	4.59	5.23	7.18	8.47	9.71	11.3	12.6
6 hour	3.92	4.47	6.17	7.30	8.37	9.79	10.9
9 hour	3.11	3.56	4.97	5.90	6.79	7.99	8.91
12 hour	2.62	3.01	4.23	5.04	5.83	6.90	7.73
18 hour	2.02	2.33	3.32	3.98	4.64	5.55	6.26
24 hour	1.67	1.93	2.75	3.32	3.89	4.68	5.31
30 hour	1.42	1.64	2.36	2.86	3.36	4.06	4.61
36 hour	1.24	1.44	2.07	2.51	2.96	3.58	4.08
48 hour	0.996	1.15	1.66	2.02	2.39	2.89	3.30
72 hour	0.718	0.826	1.18	1.44	1.71	2.06	2.35
96 hour	0.564	0.648	0.921	1.12	1.32	1.58	1.80
120 hour	0.468	0.535	0.756	0.911	1.07	1.28	1.44
144 hour	0.402	0.459	0.643	0.771	0.898	1.07	1.20
168 hour	0.354	0.404	0.562	0.669	0.775	0.919	1.03

Figure 1: IFD Design Rainfall intensity for Goodwood TAS

Site Catchments:

Pre-development:

Total site area:

Post-Development:

Post-development Impervious areas (roofs): $\approx 294m^2$ Post-development impervious areas (sealed driveway): $\approx 461m^2$ Post-development undeveloped pervious areas: $\approx 72m^2$

Pre-development entire catchment:	C = 0.55
Post-development roof:	C = 1.0
Post-development sealed driveway:	C = 0.9
Post-development pervious:	C = 0.40

≈ 827m²

Timing of Flows:

The following flow travel times have been adapted for the simulation.

Pre-development entire catchment:	TOC = 30 mins (GCC Policy)
Post-development roof:	TOC = 5 mins (Recommended for roof drainage)
•	TOC = 5 mins (Standard inlet times) TOC = 5 mins (Standard inlet times)

Calculations have been based on the Modified Rational Method for stormwater run-off:

$$Q = \frac{C \times I \times A}{3600}$$

Where:Q = Design Volumetric Flow Rate [L/s]
C = Runoff Coefficient
I = Rainfall Intensity [mm/hr] (5 minute - 5% AEP storm)
A = Sum of all equivalent areas [m²]

Pre-Existing Run off calculations

$$Q_{PSD} = \frac{(0.55 \times 827) \times 34.7}{3600} = 4.38 \,\mathrm{L/s}$$

Post-Development:

 $Q_{Post} = \frac{(1.0 \times 294 + 0.9 \times 461 + 0.40 \times 72) \times 84.8}{3600} = 17.37 \text{L/s}$

As shown above the post development flow Q_{Post} is **12.99 L/s** additional than the permissible site discharge Q_{PSD} and therefore on-site detention (OSD) is <u>required</u>. To determine the volume of storage required to reduce the post development peak discharge to the permissible site discharge Autodesk Software - Storm and Sanitary Analysis was utilised.

Due to the large differential from pervious existing surfaces to post development roof and sealed surfaces, the stormwater detention solution focuses on the use of below ground detention via a 2800L concrete detention tank and a 3000L above ground rainwater tank (refer to figure 5 and 6) connected to all drainage lines from roof and driveway pits

The model simulated both the proposed shed and proposed driveway discharging to the below ground tank system, connected to the roof area via a gravity storm water system. This below tank was simulated being fitted with a 75mm low flow orifice to restrict outflow and a DN150 overflow outlet.

The model also included the shed roof being fitted with a 3,000 L slimline detention tank, connected to the roof area via a charged stormwater system. This tank was simulated being fitted with a 40mm low flow orifice to restrict outflow.

TABLE 2: SITE OUTFLOW RESULTS

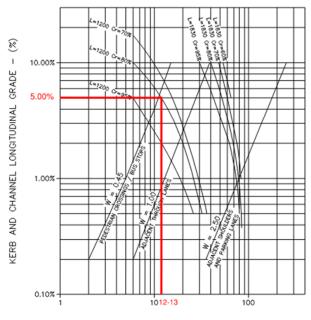
Scenario	Site Runoff (L/s)
Pre-development	4.38
Post-development (No Detention)	17.37
Post- development (Detention)	4.56 (5% AEP 5-min duration)

Table 3 further breaks down the post-development flow to demonstrate what flows are being generated and where particular catchments are draining to within the site.

TABLE 3: SITE OUTFLOW BREAKDOWN

Catchment	Detained – Yes/No	Catchment runoff (L/s)	Total Site Runoff (L/s)
Proposed Shed	Yes – Above ground 2000L detention tank	6.92	4.56
Driveway	Yes – Central below ground detention Tank	9.77	
Pervious Landscape	No Detention	0.67	

As can be seen, the peak post development run-off is still greater than the pre-development, therefore a capacity check on the kerb and gutter in Negara Crescent will be required. To assess the capacity of the kerb and gutter, the Hydraulic Capacity Graph as detailed on IPWEA LGAT TSD-RF03-v2 was utilised (see Figure 2 below).



CHANNEL FLOW AND PIT CAPACITY - (L/s)

HYDRAULIC CAPACITY ON GRADE (1220mm AND 1830mm LINTELS AT 3% CROSSFALL)

On grade inlet capture rates based on model studies. (Refer TSD design file No. JF.95.077)

FIGURE 2: IPWEA LGAT TSD-RF03-V2

With an approx. 5.00% longitudinal fall on the kerb and channel, a road crossfall off greater than 3%, Lintel width of 1200, mid-range Capture rate of 80% and allowable flow width of 1.0m, it is proposed a new 75X250 RHS channel is required and has an approximate hydraulic capacity of 12 to 13 L/s. The site outflow hydrograph can be seen below in Figure 3.

NOTES

1. Maximum flow widths:

L = Lintel Cr = Capture rate

0.45m adjacent to pedestrian crossing points and bus stops.
1.00m adjacent to traffic through lanes and

in acceleration, deceleration and left turn lanes

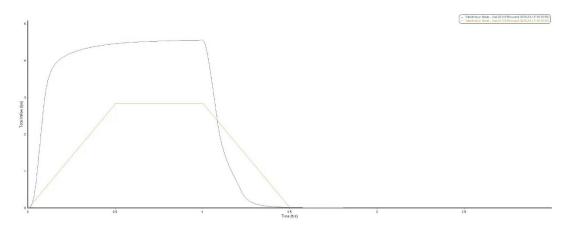
 Inlet capture rates (Cr) ignores interception by grate (assumed to be blocked by leaves). Assumes 50mm depression, 600mm long

transition, 125mm deep throat and trough below the lintel. 3. For crossfalls greater than 3% use 3% curves. For 2% crossfalls, reduce capacity by: • 25% for 1220 lintel • 50% for 1830 lintel

 Refer to 'The University Of New South Wales Water Research Laboratory – Physical Modelling Of Stormwater Side Entry Pits (628.2420994 COX)' for sealed side entry pits.

W = Flow width adjacent to kerb

2.50m adjacent to road shoulders and parking lanes.





The peak storage volume required was analysed with results as follows:

Catchment	Peak Volume (L)
Proposed Development	3867
Proposed Development	4204
Proposed Development	5178
Proposed Development	5388
Proposed Development	4878
Proposed Development	900
Proposed Development	1224
	Proposed Development Proposed Development Proposed Development Proposed Development Proposed Development Proposed Development

TABLE 4: PEAK STORAGE VOLUME

As can been seen the 20-min duration 5% AEP event is the critical in terms of storage requirement, requiring **5388L** of storage volume. However, the combination of the 3000L above ground rainwater tank and the specified below ground 2,800 L tank has more than adequate storage to handle the expected volume, with additional redundant capacity in the case of an extreme storm event.

3. DRAINAGE LAYOUT

The stormwater arrangement for the site is shown in the Fysh Design civil drawing package or figure 4 below

As discussed in the Detention section, the proposed site will be fitted with a 2800L below ground concrete detention tank, fitted with a 75mm low flow orifice outlet (refer to figure 5), as well as a DN150 overflow outlet and a surcharge pit which collects the proposed driveway and hardstand areas

It is proposed shed roof being fitted with a minimum 3000 L slimline detention tank, connected to the roof area via a charged stormwater system. This tank was simulated being fitted with a 40mm low flow orifice to restrict outflow.

By using this method, this allows for the minimum storage required on the most demanding storm event over different time durations as well as achieving lesser discharge outflows to the stormwater connection

The entire site will discharge to a new kerb connection and roadside gutter in Negara Crescent via a new 450sq. dispersion pit and 250 x 75 x 3 galvanised RHS kerb connection as per IPWEA LGAT TSD-SW29



FIGURE 4: SITE DRAINAGE LAYOUT

3a. Pipe Sizing

Pipe sizing calculations conveying stormwater from roof and hardstand catchments, rainwater tank overflow outlets have been sized to cope with a 5% AEP storm event on the Modified Rational Method and AS3500.3

$$Q = \frac{C \times I \times A}{3600}$$

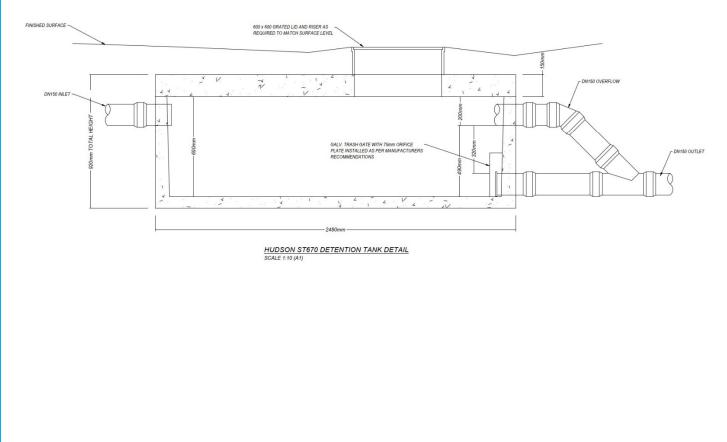
Where:Q = Design Volumetric Flow Rate [L/s]
C = Runoff Coefficient
I = Rainfall Intensity [mm/hr] (5 minute - 5% AEP storm
A = Sum of all equivalent areas [m²]

Pipework Material PVC with Colebrook-White roughness coefficient K = 0.015 (From AS3500.3 Table 5.4.11.2)

Minimum grade of pipework of 1% (HG 1:100)

Pipe size selected from AS3500.3 Figure 5.4.11.2(a)

 $Q_{Post} = \frac{(1.0 \times 294 + 0.9 \times 461 + 0.30 \times 30) \times 84.8}{3600} = 17.20 L/s = DN150 PVC$



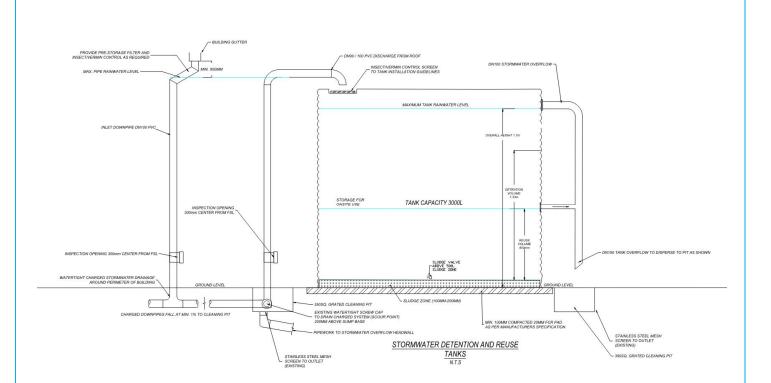


FIGURE 4: ABOVE GROUND 2200L DETENTION TANK DETAIL

4. TREATMENT

In partnership with proprietary stormwater treatment supplier Ocean Protect, Model for Urban Stormwater Improvement Conceptualisation (MUSIC Version 6.3.0) will be used to model the site roof drainage and impervious areas with effectiveness of various treatment devices to achieve the stormwater quality targets outlined in the State Stormwater Strategy (2010) of:

- An 80% reduction in the average annual load of total suspended solids (TSS)
- An 45% reduction in the average annual load of total phosphorous (TP)
- An 45% reduction in the average annual load of total nitrogen (TN)
- 90% Gross Pollutant Reduction

Figure 6 displays a site area breakdown modeled within the MUSIC software and the system meeting the required treatment targets.

As shown in Figure 6, Ocean Protect has proposed the use of a JellyFish JF900-1-1 (686mm cartridges) to treat the stormwater run-off from the development. MUSIC modelling can be provided to Council to ensure compliance with treatment targets once the detailed design has been completed. This system is to be installed as close as possible to the property connection to ensure as much runoff as possible is treated.



FIGURE 6: MUSIC MODEL

6. MAINTENANCE

Maintenance requirements for grated stormwater pits:

Regular inspections and clean outs of grated stormwater pits when required. This should be performed every 6 months to annually, dependant on site conditions.

Maintenance requirements for below ground detention tanks:

Regular monthly inspection of the low flow orifice outlet and galvanized trash guard for foreign debris to prevent blockage, ensuring sludge zones of the tank does not reach orifice height. External visual inspection is to be performed annually, checking the overall condition of tank and pipework.

Vacuum tank silt and sediment from detention tank and pits approximately every 4-5 years

Maintenance requirements for Ocean Protect treatment system.

Maintenance of the OceanGuard is simple, effective and seldom requires confined space entry or specialized equipment, often being completed by hand without the need for vacuum equipment. Simply remove the OceanGuard from the pit with the tags provided and invert the bag into a waste bin. Inspect the liner and brush by hand or spray with a pressure washer if required to rejuvenate the filtration bag. Record the information and replace the filtration bag. The Ocean Guard® system should be inspected at regular intervals from 1-2 months during the first year of installation to ensure optimum performance. The frequency at which the OceanGuard will need to be maintained will depend on site activities, land uses, catchment area and this size of OceanGuard installed, 1- 6 times annually (3-4 typ.).

For further information please refer to the OceanGuard Operations and Maintenance Manual.

7. CONCLUSION

This report has demonstrated that the proposed development 115 Howard Road Goodwood complies with the stormwater quantity and quantity conditions of the Glenorchy City Council Stormwater Policy.

Note:

- This report assumes the Council stormwater main or roadside kerb has capacity for permissible site discharge.
- It is the responsibility of Council to assess their infrastructure and determine the impact (if any) of altered inflows into their stormwater network.

Please contact <u>cfysh@fyshdesign.com.au</u> if you require any additional information.

Yours sincerely

Chris Fysh

Director

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Email: cfysh@fyshdesign.com.au



241011 115 HOWARD ROAD REV 3 15/01/2025